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1

BUILDING MATERIALS AND STRUCTURES

ČASOPIS ZA ISTRAŽIVANJA U OBLASTI MATERIJALA I KONSTRUKCIJA
JOURNAL FOR RESEARCH OF MATERIALS AND STRUCTURES



DRUŠTVO ZA ISPITIVANJE I ISTRAŽIVANJE MATERIJALA I KONSTRUKCIJA SRBIJE
SOCIETY FOR MATERIALS AND STRUCTURES TESTING OF SERBIA

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PROCENA ČVRSTOĆE BETONA PRI PRITISKU, KORIŠĆENJEM VEŠTAČKIH NEURONSKIH MREŽA

ESTIMATION OF CONCRETE COMPRESSIVE STRENGTH USING ARTIFICIAL NEURAL NETWORK

Srđan KOSTIĆ
Dejan VASOVIĆ

ORIGINALNI NAUČNI RAD
ORIGINAL SCIENTIFIC PAPER
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1 UVOD

Analiza čvrstoće betona pri pritisku predstavlja jedan od primarnih zadataka laboratorijskih ispitivanja za različite potrebe inženjerske prakse, pre svega u zgradarstvu, tunelogradnji, putarstvu, pri izgradnji brana i mostova, kao i pri izvođenju različitih podzemnih i površinskih konstrukcija u rudarstvu. Zavisno od zahteva projekta, ispitivanje čvrstoće betona izvodi se za uzorke različite starosti s promenljivim vodocementnim faktorom, raznim tipovima i količinom aditiva (leteći pepeo, silikatna prašina, metakaolin i mlevena granulirana zgora iz visokih peći kao mineralni aditivi, odnosno plastifikatori, različiti akceleratori i retarderi, kao hemijski aditivi), te u različitim spoljašnjim uslovima ugradnje betona (s naglaskom na otpornost betona pri izlaganju mrazu). Pri tome, pouzdanost dobijenih rezultata najčešće predstavlja direktnu funkciju broja ispitanih uzoraka, tj. veći broj ispitanih uzoraka betona doprinosi pouzdanijem određivanju njegovih svojstava. Međutim, sredstva predviđena programom istraživanja, u najvećem broju slučajeva, nisu dovoljna za analizu brojnih uzoraka, već se najčešće pristupa interpolaciji malog, vrlo često i nedovoljnog broja podataka ispitivanja. U tom smislu, modeli za procenu čvrstoće betona predstavljaju posebno važnu tehniku koja omogućava utvrđivanje relacije između zrelosti betona i njegove čvrstoće,

1 INTRODUCTION

Analysis of concrete compressive strength represents one of the primary tasks in laboratory studies for different needs of engineering praxis, including architectural engineering, tunnelling, road engineering, construction of dams and bridges, and for the purpose of surface and underground mining activities. Depending on the Project demand, concrete compressive strength is examined for the specimens of different age and with distinct w/c ratio, for different types and amounts of additives (flying ash, silica fume, metakaolin and ground granulated blast furnace slag, as mineral additives, and plasticizers, different accelerators and retarders, as chemical additives). In this case, reliability of the obtained results regularly represents a direct function of the number of examined concrete samples. In other words, the larger the number of analyzed specimens, the more precise their properties are determined. However, only a small part of the project funding is used for laboratory analyzes, which is often scarce for conducting the analysis of larger number of samples. Instead, the analysis is often based on the approximation of small and insufficient data. Therefore, existing models for estimation of concrete compressive strength have valuable importance, enabling us to determine the relation between the maturity of concrete and its compressive

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odnosno ocenu razvoja čvrstoće betona s vremenom na bazi relativno malog broja ispitanih uzoraka. Jedan od prvih uspješnih postupaka procene čvrstoće betona pri pritisku dao je Plouman [1], koji je vezu između čvrstoće i zrelosti po Solu izrazio u obliku logaritamske funkcije. S druge strane, Bernhart [2] je pokazao da je brzina razvoja relativne čvrstoće betona proporcionalna veličini nehidratiranog dela betona, koju je uveo preko konstante proporcije k . Karino [3] je takođe predložio ocenu čvrstoće pri pritisku uzoraka betona, pod pretpostavkom da očvršćavanje betona počinje tek nakon određenog vremena od ugradnje betona. Yi i dr. [4] u svoju jednačinu za procenu čvrstoće betona pri pritisku uveli su efekat difuzione ljuske, konstantu brzine, graničnu čvrstoću i reakcioni koeficijent.

Uprkos činjenici da ovi konvencionalni modeli daju ocenu pritisne čvrstoće betona sa zadovoljavajućom tačnošću za potrebe inženjerske prakse, razvoj novih smeša betona, s različitim tipovima i količinom aditiva, povećava broj sastavnih elemenata betona, što otežava uspostavljanje jasnih veza između različitih komponenta. Iz tog razloga, tokom poslednjih godina, sve češća je primena veštačkih neuronskih mreža za potrebe modelovanja različitih svojstava betona, poput skupljanja pri isušivanju [5], trajnosti betona [6], čvrstoće normalnog betona i betona visoke čvrstoće pri pritisku [7–12], konsistencije betona s metakaolinom i letećim pepelom [13–14], mehaničkog ponašanja betona na visokim temperaturama [15], kao i dugotrajnog efekta letećeg pepela i silikatne prašine na čvrstoću betona pri pritisku [16]. Glavna prednost primene veštačkih neuronskih mreža u odnosu na standardne konvencionalne prediktore [1–4], leži u mogućnosti analize čvrstoće velikog broja uzoraka betona s različitim vodocementnim faktorom, uključujući i efekat izlaganja dejstvu mraza. Za razliku od veštačkih neuronskih mreža, konvencionalnim prediktorima procenjuje se razvoj čvrstoće pri pritisku uzoraka betona istog sastava (jednak vodocementni faktor), koji su negovani u izotermalnim uslovima.

Pored navednih konvencionalnih modela i veštačkih neuronskih mreža, neretko se koriste i drugi modeli procene čvrstoće betona pri pritisku, koji se zasnivaju na razmatranju efekta različitih proporcija vode, cementa i agregata [17–18], odnosno koji koriste sisteme na bazi adaptivne mreže [19–21] ili fazi logike [22–24].

U ovom radu razvijen je model procene čvrstoće betona pri pritisku na bazi veštačkih neuronskih mreža, korišćenjem rezultata eksperimentalnih ispitivanja, u zavisnosti od četiri kontrolna faktora: vodocementni faktor, starost betona, broj ciklusa zamrzavanja/otkravlivanja i količina superplastifikatora.

2 SVOJSTVA BETONA

2.1 Cement

Za pripremu uzoraka betona za ispitivanje korišćen je CEM I normalni Portland cement (PC 42,5 N/mm²) sa

strength, providing, in that way, an evaluation of compressive strength development with time on the basis of relatively small number of examined samples. One of the first successful prediction models was provided by Plowman [1], who expressed the relationship between strength and maturity by Saul as a natural logarithmic function. Soon afterwards, Bernhardt [2] showed that relative strength development ratio of concrete is proportional to the size of unhydrated portion of the concrete and introduced rate constant k . Carino [3] also suggested the equation of prediction of concrete compressive strength under the assumption that the hardening of concrete starts at a certain time after the concrete placement time. Yi et al. [4] incorporate the effect of diffusion shell, rate constant, limiting strength and reaction coefficient, as functions of curing temperature, in the equation of concrete strength prediction.

Despite the fact that previous conventional models give reasonable prediction accuracy for engineering purposes in reference to concrete compressive strength, development of new concrete mixtures, with different types and percentage of additives, increase the number of concrete constituents, thus, making harder to obtain reliable results among various concrete components. Therefore, in recent years, artificial neural networks (ANN) have been used for the purpose of modelling different properties of concrete, such as drying shrinkage [5], concrete durability [6], compressive strength of normal concrete and high performance concrete [7–12], workability of concrete with metakaolin and fly ash [13–14], mechanical behaviour of concrete at high temperatures [15] and long term effect of fly ash and silica fume on compressive strength [16]. The main advantage of ANN approach over the standard conventional predictors [1–4] lies in the possibility to examine the compressive strength of large number of concrete specimens with different w/c ratio, including the effect of exposure to various freeze/thaw cycles. Opposite to the ANN approach, conventional predictors estimate the development of compressive strength of concrete specimens of the same properties (equal w/c ratio) cured at isothermal conditions.

Besides conventional models and the ANN approach, there are other types of models which are frequently used for prediction of compressive strength. The first of them is based on the combination of input variables, water, cement and aggregates [17–18], while the second approach is using adaptive network-based fuzzy inference system [19–21] and fuzzy logic techniques [22–24].

In present paper, the ANN model is developed for estimation of concrete compressive strength based on the results of a series of experiments. The present research is focused on compressive strength of concrete samples, depending on four main factors: w/c ratio, age, number of freeze/thaw cycles and amount of superplasticizer.

2 PROPERTIES OF MATERIALS

2.1 Cement

The examined concrete specimens were made of CEM I normal Portland cement (PC 42.5 N/mm²) with

2.3 Plastifikator

Superplastifikator (SP) tipa melamin korišćen je radi održavanja konsistencije i sleganja sveže betonske smeše. Količina dodatog superplastifikatora za različite betonske smeše data je u Tabeli 4. Dodavanjem plastifikatora proporcionalno je smanjivana količina vode.

Tabela 4. Proporcije smeša betona i njihova konsistencija
Table 4. Concrete mixture proportions and consistency

Uzorak br. Sample No.	C (kg)	A (kg)	VC	SP (%)	Sleganje Slump (cm)	Vebe (s)	Tečenje Flow (cm)
D1	350	1930	0,45	2,0	8	4	36
D2	350	1930	0,40	2,0	5	5,5	32
D3	350	1930	0,50	1,4	17	1,5	57
D4	350	1930	0,55	1,4	25,5	0	67
D5	350	1930	0,35	4,0	2	10	25

2.4 Priprema uzoraka

Smeše betona pravljenе su u laboratorijskom mikseru tipa „Eirich”, s periodom mešanja od tri minuta za sve smeše. Za testiranje su pripremljeni kockasti uzorci betona (100x100mm). Livenje betona izvedeno je na vibracionom stolu sve do potpune konsolidacije. Konsistencija svežeg betona određivana je pomoću testa sleganja [25], testa po Vebeu [26] i testa tečenja [27].

3 POSTUPAK TESTIRANJA

Nakon što je beton izliven u metalne kalupe, uzorci su ostavljeni na sobnoj temperaturi ($+20^{\circ}\pm 2^{\circ}\text{C}$) sa 90 – 95% RH. Nakon 24^{h} uzorci su izvađeni iz kalupa i potopljeni u vodu na istoj temperaturi ($+20^{\circ}\text{C}$) sledećih šest dana. Sedmog dana, četiri serije od osam serija uzoraka betona izlagane su mrazu. Čvrstoća pri pritisku određivana je nakon 50 i 100 ciklusa zamrzavanja/otkravljivanja (jedan ciklus podrazumeva izlaganje uzorka mrazu u trajanju od 4^{h} u komori na temperaturi $-20^{\circ}\pm 2^{\circ}\text{C}$, a potom se uzorak izlaže sobnoj temperaturi od $20^{\circ}\pm 2^{\circ}\text{C}$ u vodi u trajanju od 4^{h}). Nakon toga, izmerena vrednost čvrstoće poređena je sa čvrstoćom kontrolne grupe uzoraka (koji su neprekidno negovani u vodi na temperaturi $20^{\circ}\pm 2^{\circ}\text{C}$) za ekvivalentnu starost [28–29]. Čvrstoća na pritisak i nasipna gustina određeni su prema važećim standardima [30–31]. Čvrstoća pri pritisku uzoraka betona određivana je pomoću „Amsler” hidrauličke prese kapaciteta 2000 kN, pri brzini pritiska od 0,4 MPa/s.

4 EKSPERIMENTALNI REZULTATI

Ekperimentalno dobijeni rezultati jasno ukazuju na uticaj vodocementnog faktora na čvrstoću betona pri pritisku (Tabela 5). Uzorci betona s nižim vodocementnim faktorom pokazuju mnogo veću čvrstoću pri pritisku, koja je određena granulometrijskim sastavom agregata i

2.3 Plasticizer

A melamine-type superplasticizing admixture (SP) was used at various amounts to maintain slump and workability of fresh concrete mixture. The amount of SP used in the different concrete mixtures is given in Table 4. The amount of water was decreased for the amount of SP added.

2.4 Preparation of specimens

Concrete was made in a laboratory counter-current concrete mixer (type “Eirich”). Mixing period was 3 minutes for all mixtures. Cubic samples (100x100mm) were made for testing. Casting was performed at vibrating table until a complete consolidation was achieved. Consistency of fresh concrete was measured by applying the slump test [25], Vebe test [26] and flow test [27].

3 TEST PROCEDURE

After the concrete was casted in metal moulds, samples were left at ambient room temperature ($20^{\circ}\pm 2^{\circ}\text{C}$) with 90 – 95% RH. After 24 h the concrete samples were demoulded and soaked in the water at the same temperature (20°C) for the next six days. After seven days, four out of eight series of the concrete samples were exposed to freezing and thawing. Compressive strength was determined after 50 and 100 cycles (one cycle lasted for 4^{h} in environmental chamber at $-20^{\circ}\pm 2^{\circ}\text{C}$ and 4^{h} soaked in water at $20^{\circ}\pm 2^{\circ}\text{C}$). Afterwards, measured strength was compared with the strength of control group of specimens (continually cured in water at $20^{\circ}\pm 2^{\circ}\text{C}$) at the equivalent age [28-29]. The compressive strength and bulk density of hardened concrete were tested according to the existing standards [30-31]. Compressive strength measurements were carried out using “Amsler” hydraulic press with a capacity of 2000 kN and 0.4 MPa/s loading rate.

4 EXPERIMENTAL RESULTS

The obtained results clearly indicate the impact of w/c ratio on compressive strength of concrete (Table 5). Samples of concrete with lower w/c ratio have higher compressive strength which is determined by the aggregate grading and amount of cement in the mixture.

5 PROCENA ČVRSTOĆE PRI PRITISKU BETONA

U drugoj fazi istraživanja, nakon eksperimentalnog dela, pristupilo se razvoju modela na bazi veštačkih neuronskih mreža, s četiri ulazna podatka i samo jednim izlaznim podatkom (Tabela 6). Sličan pristup je već korišćen u prethodnim istraživanjima [18, 21, 32–33].

5 ESTIMATION OF CONCRETE COMPRESSIVE STRENGTH

In the second phase of the research, after performing experimental tests, we turn to development of a neural network model, with four input parameters and a single output unit (Table 6). Similar approach was already used in [18, 21, 32–33].

Tabela 6. Raspon vrednosti ulaznih i izlaznih podataka za obučavanje neuronske mreže
Table 6. Input-output parameters for the ANN training and their range

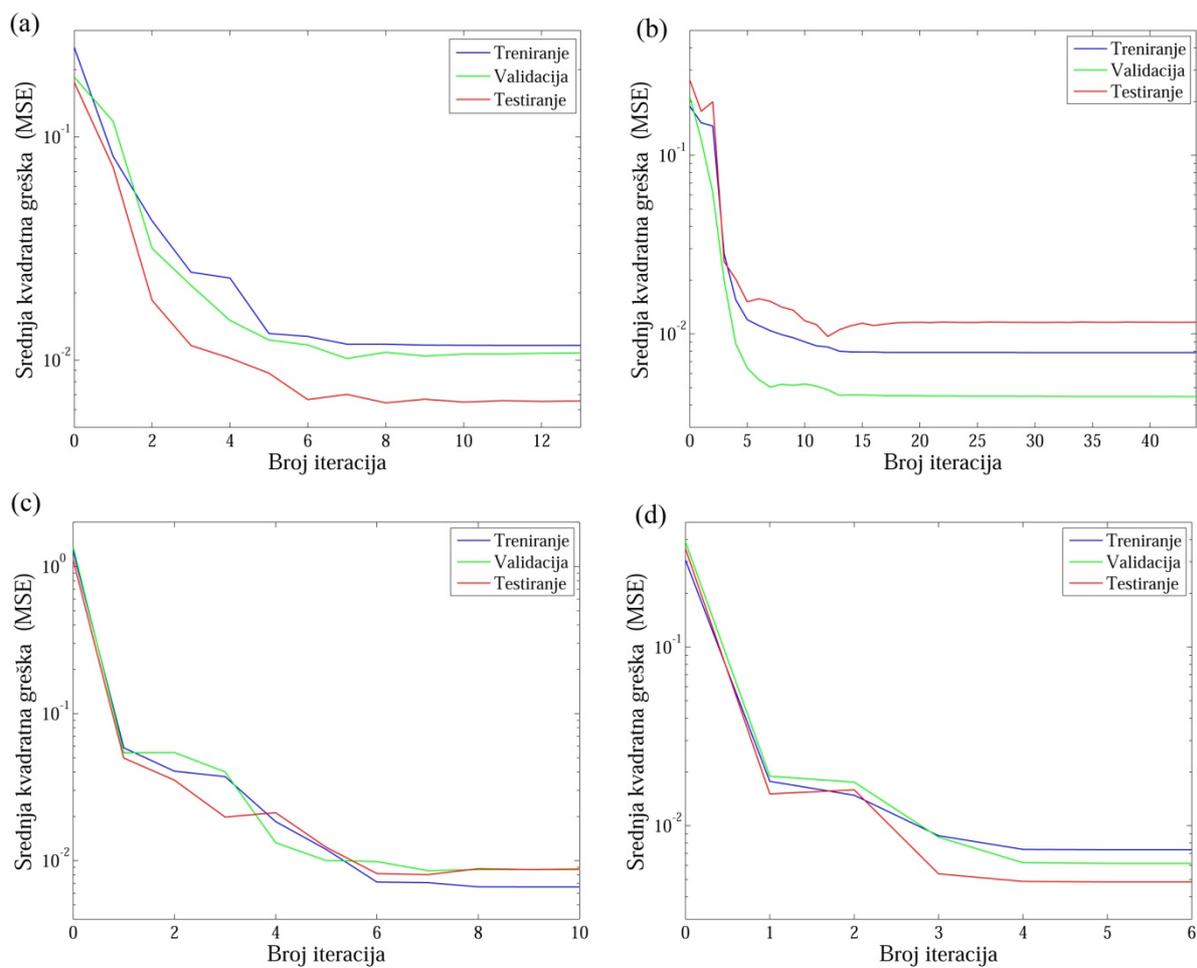
Podaci Type of data	Parametar Parameter	Raspon vrednosti Range
Ulazni Inputs	vodocementni faktor (%) w/c ratio (%)	0.35–0.55
	starost (dani) age (days)	7–32
	količina superplastifikatora (%) amount of superplasticizer (%)	1.4–4
	broj ciklusa zamrzavanja/otkravlivanja number of freeze/thaw cycles	0–100
Izlazni Output	čvrstoća na pritisak (MPa) compressive strength (MPa)	21.4–55

Na osnovu predloga Rumelharta i dr. [34], Lipmana [35] i Sonmeza i dr. [36], analizirana je veštačka neuronska mreža sa samo jednim skrivenim slojem, dok je broj jedinica u skrivenom sloju određen korišćenjem heurističkih obrazaca [36]. Kao što se u Tabeli 7 može videti, na osnovu broja ulaznih i izlaznih podataka, broj jedinica u skrivenom sloju je u rasponu od 1 do 12. U ovom slučaju, pristupilo se ispitivanju veštačkih neuronskih mreža s jednom jedinicom, tri jedinice, te osam i dvanaest jedinica u skrivenom sloju, radi određivanja modela s najpreciznijom procenom čvrstoće pri pritisku betona.

Following the suggestion of Rumelhart et al. [34], Lippmann [35] and Sonmez et al. [36] one hidden layer was chosen in present study, while the number of hidden neurons was determined using heuristics [36]. As it is clear from Table 7, the number of neurons that may be used in the hidden layer varies between 1 and 12. In present study, the number of hidden neurons was selected as 1, 3, 8 and 12 separately to establish the most effective ANN architecture.

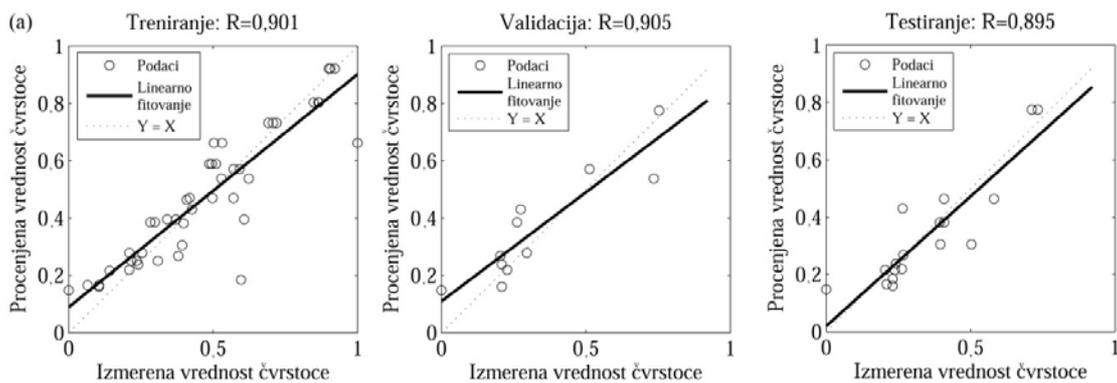
Tabela 7. Heuristički obrasci za određivanje broja jedinica u skrivenom sloju
(N_i : broj ulaznih jedinica, N_0 : broj izlaznih jedinica)
Table 7. The heuristics used for the number of neurons in hidden layer
(N_i : number of input neurons, N_0 : number of output neurons)

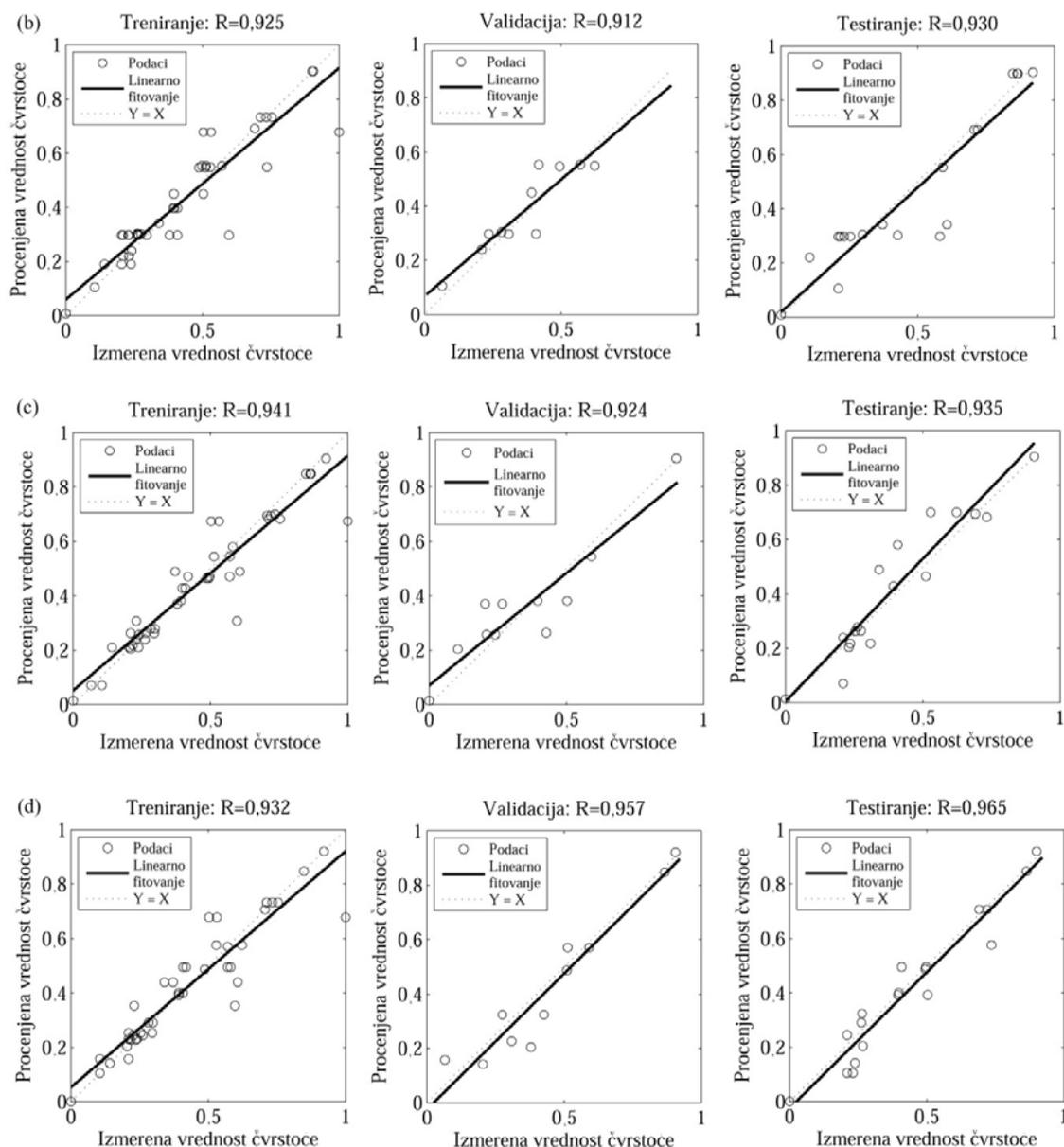
Heuristički obrazac Heuristic	Calculated number of neurons for this study
$\leq 2 \times N_i + 1$	≤ 9
$3 \times N_i$	12
$(N_i + N_0) / 2$	2,5 (3)
$\frac{2 + N_0 \times N_i + 0.5N_0 \times (N_0^2 + N_i) - 3}{N_i + N_0}$	1,1 (1)
$2N_i / 3$	2,7 (3)
$\sqrt{(N_i + N_0)}$	2,2 (2)
$2N_i$	8



Slika 1. Srednja kvadratna greška u funkciji broja iteracija za treniranje, validaciju i testiranje, za različiti broj jedinica u skrivenom sloju: (a) 1, (b) 3, (c) 8 i (d) 12

Figure 1. MSE versus the number of epochs for training, validation and testing data, with different number of hidden neurons: (a) 1, (b) 3, (c) 8 and (d) 12.





Slika 2. Poređenje skaliranih procenjenih i izmerenih vrednosti čvrstoće pri pritisku betona za podatke iz ciklusa treniranja, validacije i testiranja, korišćenjem neuronske mreže s različitim brojem jedinica u skrivenom sloju: (a) 1, (b) 3, (c) 8 i (d) 12

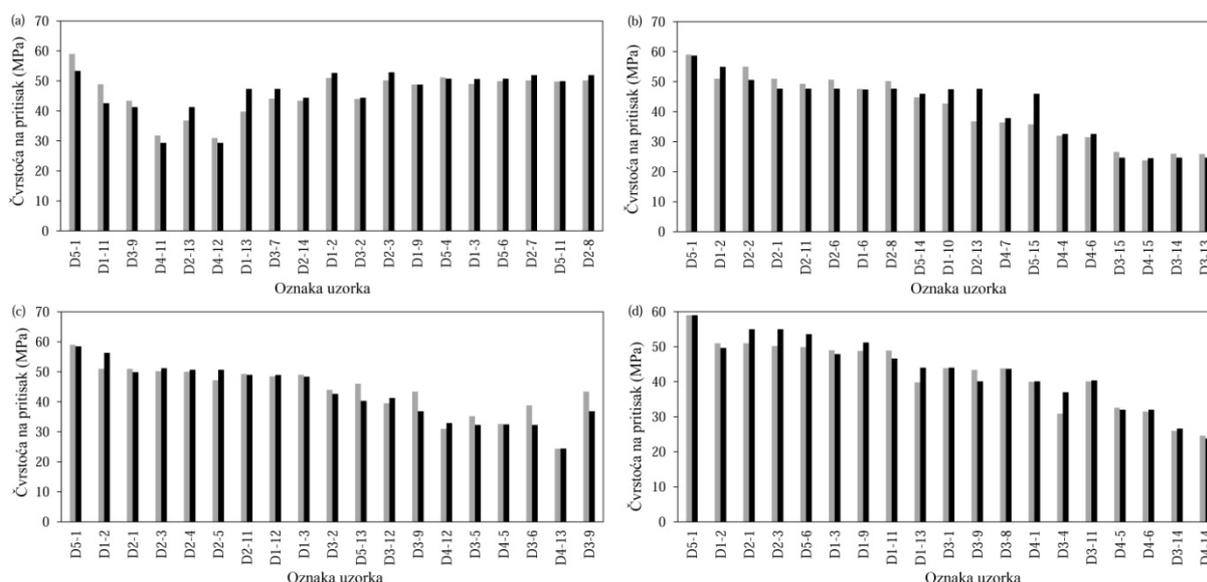
Figure 2. Comparison of the scaled estimated and measured values of concrete compressive strength showing training, validation and testing set, for the following number of hidden nodes: (a) one, (b) three, (c) eight and (d) twelve

6 OCENA USPEŠNOSTI MODELA

Ocena preciznosti predloženih modela veštačkih neuronskih mreža s različitim brojem jedinica u skrivenom sloju može dalje biti izvedena na osnovu poređenja njihovih neskaliniranih vrednosti (iz ciklusa testiranja) sa eksperimentalnim rezultatima (slika 3). Jasno je da se u svim ispitivanim slučajevima veštačkim neuronskim mrežama daje pouzdana procena čvrstoće pri pritisku betona. Neophodno je naglasiti da se na slici 3 različiti uzorci koriste za testiranje u svakom od ispitivanih slučajeva s različitim brojem skrivenih jedinica, zbog nasumično odabranih početnih uslova.

6 EVALUATION OF MODEL PERFORMANCE

On the basis of the proposed ANN model with different number of hidden nodes, their precision of estimation could be further evaluated by comparing the unscaled predicted values (testing data) with experimental results (Figure 3). It is clear that in most of the examined cases, ANN gives reasonable value of concrete compressive strength. One should note that different samples are used for testing in each of the examined cases with different number of hidden nodes, due to random initial conditions.



Slika 3. Poređenje neskaliranih procenjenih i izmerenih vrednosti čvrstoće na pritisak uzoraka betona, korišćenjem neuronske mreže s različitim brojem jedinica u skrivenom sloju: (a) 1; (b) 3; (c) 8 i (d) 12. Sivim su označene izmerene vrednosti, a crnim procenjene vrednosti čvrstoće na pritisak betona. Različiti uzorci koriste se za testiranje u svakom od ispitivanih slučajeva s različitim brojem skrivenih jedinica, zbog nasumično odabranih početnih uslova

Figure 3. Comparison of measured and estimated compressive strength of concrete specimens by using ANN models with different number of hidden neurons: (a) 1; (b) 3; (c) 8 and (d) 12. Grey bars stand for the measured values; black bars denote estimated values of compressive strength. Different samples are used for testing in each of the examined cases with different number of hidden nodes, due to random initial conditions

Preciznost razvijenih modela s različitim brojem skrivenih jedinica može se dalje oceniti izračunavanjem vrednosti različitih standardnih statističkih grešaka, datih u Tabeli 8 [40].

Performances of the developed models with different number of hidden nodes could be further evaluated using different standard statistical error criteria given in Table 8 [40].

Tabela 8. Pregled statističkih grešaka korišćenih za ocenu uspešnosti predloženog modela*
Table 8. Preview of statistical error parameters used for models' evaluation*

Statistička greška Statistical parameter	Jednačina Equation
Srednja apsolutna greška (MAPE) Mean Absolute Percentage Error (MAPE)	$MAPE = \frac{1}{n} \times \left[\sum_{i=1}^n \left \frac{t_i - x_i}{t_i} \right \right] \times 100$
Varijansa relativne vrednosti apsolutne greške (VARE) Variance Accounted For (VAF)	$VARE = \frac{1}{n} \times \left[\sum_{i=1}^n \left(\left \frac{t_i - x_i}{t_i} \right - \text{mean} \left \frac{t_i - x_i}{t_i} \right \right)^2 \right] \times 100$
Medijana (MEDAE) MEDian Absolute Error (MEDAE)	$MEDAE = \text{median}(t_i - x_i)$
Sračunata varijansa (VAF) Variance Absolute Relative Error (VARE)	$VAF = \left[1 - \frac{\text{var}(t_i - x_i)}{\text{var}(t_i)} \right] \times 100$

* t_i predstavlja izmerenu vrednost čvrstoće pri pritisku, a x_i predstavlja procenjenju vrednost čvrstoće pri pritisku.

* t_i represents measured value of compressive strength, while x_i denotes predicted value of compressive strength.

Izračunate statističke greške za veštačke neuronske mreže s različitim brojem jedinica u skrivenom sloju date su u Tabeli 9. Jasno je da veštačka neuronska mreža sa dvanaest jedinica u skrivenom sloju daje najmanje vrednosti srednje apsolutne greške (MAPE), varijanse relativne vrednosti apsolutne greške (VARE) i medijane (MEDAE), a najveću vrednost sračunate varijanse (VAF) u odnosu na neuronske mreže s jednom jedinicom, tri jedinice i osam jedinica u skrivenom sloju.

Calculated values of statistical errors for neural networks with different number of hidden nodes are given in Table 9. It is clear that ANN model with twelve hidden nodes has the lowest values of MAPE (Mean Absolute Percentage Error), VARE (Variance Absolute Relative Error) and MEDAE (MEDian Absolute Error), and the highest value of VAF (Variance Accounted For), in comparison to the ANN models with one, three or eight hidden nodes.

Tabela 9. Statističke greške u proceni čvrstoće pri pritisku uzoraka betona korišćenjem neuronske mreže s različitim brojem jedinica u skrivenom sloju

Table 9. Statistical errors of the ANN models with different number of hidden nodes for estimation of concrete compressive strength

Veštačka neuronska mreža ANN model	Statističke greške Statistical errors			
	MAPE	VARE	MEDAE	VAF
Broj jedinica u skrivenom sloju No. of hidden nodes				
1	5,47	5,44	1,74	90,77
3	7,31	7,25	1,65	81,81
8	5,74	5,71	1,35	88,21
12	4,61	4,59	1,10	92,80

7 ZAKLJUČAK

U radu je predloženo nekoliko modela veštačkih neuronskih mreža za procenu čvrstoće pri pritisku betona, korišćenjem rezultata opita na 75 uzoraka s različitim vodocementnim faktorom i količinom superplastiifikatora. Uzorci betona izlagani su različitom broju ciklusa zamrzavanja i otkravljanja, a njihova čvrstoća na pritisak određivana je nakon 7, 20 i 32 dana. Eksperimentalni rezultati ukazuju na to da sa smanjenjem vodocementnog faktora, čvrstoća pri pritisku betona raste do vrednosti koja je određena granulometrijskim sastavom agregata i količinom cementa u betonskoj smeši. Dalje smanjenje vodocementnog faktora dovodi do smanjenja pritisne čvrstoće, s obzirom na to što betonska smeša gubi konsistenciju. S druge strane, smenjivanje ciklusa zamrzavanja i otkravljanja takođe smanjuje čvrstoću pri pritisku, naročito pri visokim vrednostima vodocementnog faktora. Uzorci betona sa superplastiifikatorom izloženi zamrzavanju pokazuju povećanje čvrstoće pri pritisku čak nakon 50 i 100 ciklusa zamrzavanja/otkravljanja.

Na bazi ovako dobijenih eksperimentalnih rezultata, predloženo je nekoliko modela veštačkih neuronskih mreža, s različitim brojem jedinica u skrivenom sloju, određenih na osnovu broja ulaznih i izlaznih jedinica. U svim modelima primenjena je veštačka neuronska mreža s prostiranjem signala unapred i s propagacijom greške unazad, korišćenjem Levenberg-Marquardt algoritma obučavanja. Rezultati izvedenog istraživanja pokazali su da neuronska mreža sa 12 jedinica u skrivenom sloju daje najprecizniju procenu pritisne čvrstoće betona, s najmanjom vrednošću statističkih grešaka MAPE, VARE i MEDAE, i najvećom vrednošću sračunate varijanse (VAF).

7 CONCLUSIONS

In present paper, the ANN model for estimation of concrete compressive strength is proposed using the experimental results on 75 specimens with different w/c ratio and different amount of superplasticizer. The concrete samples were exposed to different number of freeze/thaw cycles, while their compressive strength was determined at different age (7, 20 and 32 days). Experimental results indicate that by decreasing the w/c ratio, the compressive strength increases up to some level, which is determined by the aggregate grading and amount of cement in the mixture. Further decrease of w/c ratio also decreases compressive strength because the concrete mixture is losing workability. On the other hand, freezing and thawing cycles also decreases the concrete strength, especially at higher w/c ratios. Concrete samples with SP exposed to freezing show increase in strength even after 50, and more clearly after 100 cycles.

On the basis of the obtained experimental results, several ANN models were developed, using different number of hidden nodes, which were determined according to the number of input and output nodes. In all the examined cases, a three layer feed-forward back-propagation network with Levenberg-Marquardt learning algorithm was used. The performed research showed that the ANN model with twelve hidden nodes provides the most accurate estimation of concrete compressive strength, comparable to the experimental results. Further analysis indicated that neural network with 12 hidden nodes has the lowest values of MAPE, VARE and MEDAE, and the highest value of VAF, confirming this model as the most precise one for estimation of concrete compressive strength.

Međutim, uprkos visokoj preciznosti predložene veštačke neuronske mreže, jedno od glavnih ograničenja ove analize predstavlja relativno jednostavan sastav ispitivanih betonskih smeša. Svakako bi važno bilo da se, u okviru budućih analiza, u obzir uzmu i uzorci betona s različitim savremenim aditivima (leteći pepeo, zeolit, topioničarska zgura i dr.), što bi dovelo do poboljšanja predloženog modela neuronske mreže i njegove veće primenljivosti u svakodnevnoj praksi. Sa stanovišta konstrukcije veštačke neuronske mreže, detaljnija analiza pouzdanosti modela s različitim algoritimima učenja i propagacije greške sigurno bi doprinela boljem razumevanju mogućnosti primene ovih metoda za procenu čvrstoće betona pri pritisku.

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REZIME

PROCENA ČVRSTOĆE BETONA PRI PRITISKU, KORIŠĆENJEM VEŠTAČKIH NEURONSKIH MREŽA

Srđan KOSTIĆ
Dejan VASOVIĆ

U radu se daje procena čvrstoće betona pri pritisku, primenom veštačkih neuronskih mreža s prostiranjem signala unapred i propagacijom greške unazad. Obučavanje mreže sprovodi se korišćenjem Levenberg-Marquardt algoritma obučavanja za četiri različite arhitekture neuronskih mreža, s jednom jedinicom, tri jedinice, te osam i dvanaest jedinica u skrivenom sloju, radi odbacivanja efekta „pretreniranja”. Treniranje, validacija i testiranje neuronskih mreža izvodi se na osnovu rezultata eksperimentalnog ispitivanja čvrstoće pri pritisku na 75 uzoraka betona, s različitim vodocementnim faktorom i količinom superplastifikatora tipa melamina. Ispitivani uzorci betona izlagani su različitim ciklusima zamrzavanja/otkravlivanja, a njihova čvrstoća pri pritisku određivana je nakon 7, 20 i 32 dana. Dobijeni rezultati ukazuju na to da neuronska mreža s dvanaest jedinica u skrivenom sloju daje ocenu čvrstoće zadovoljavajuće tačnosti u poređenju sa eksperimentalno dobijenim podacima ($R \approx 0,97$, $MSE = 0,005$). Rezultati izvedene analize dodatno su potvrđeni sračunavanjem vrednosti standardnih statističkih grešaka: najmanjom vrednošću srednje apsolutne greške (MAPE), varijanse relativne vrednosti apsolutne greške (VARE) i medijane (MEDAE), kao i najvećom vrednošću sračunate varijanse (VAF) za izabranu arhitekturu neuronske mreže.

Cljučne reči: čvrstoća betona, vodocementni faktor, superplastifikator, zamrzavanje/otkravlivanje, starost, veštačka neuronska mreža

SUMMARY

ESTIMATION OF CONCRETE COMPRESSIVE STRENGTH USING ARTIFICIAL NEURAL NETWORK

Srđan KOSTIĆ
Dejan VASOVIĆ

In present paper, concrete compressive strength is evaluated using back propagation feed-forward artificial neural network. Training of neural network is performed using Levenberg-Marquardt learning algorithm for four architectures of artificial neural networks, one, three, eight and twelve nodes in a hidden layer in order to avoid the occurrence of overfitting. Training, validation and testing of neural network is conducted for 75 concrete samples with distinct w/c ratio and amount of superplasticizer of melamine type. These specimens were exposed to different number of freeze/thaw cycles and their compressive strength was determined after 7, 20 and 32 days. The obtained results indicate that neural network with one hidden layer and twelve hidden nodes gives reasonable prediction accuracy in comparison to experimental results ($R=0.965$, $MSE=0.005$). These results of the performed analysis are further confirmed by calculating the standard statistical errors: the chosen architecture of neural network shows the smallest value of mean absolute percentage error (MAPE), variance absolute relative error (VARE) and median absolute error (MEDAE), and the highest value of variance accounted for (VAF).

Keywords: concrete strength, w/c ratio, superplasticizer, freezing/thawing, age, artificial neural network

EVALUATION OF STRUCTURAL RELIABILITY USING SIMULATION METHODS

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1 INTRODUCTION

Reliability is defined in EN 1990 (Eurocode 0 – Basis of structural design, [1]) as “the ability of a structure or a structural member to fulfill the specified requirements, including the design working life, for which it has been designed”. Reliability is usually expressed in probabilistic terms and it covers safety, serviceability and durability of a structure (see [1]). The following relation between the “probability of failure” P_f and the index of reliability β is given in EN 1990, Annex C:

$$P_f = \Phi(-\beta) \quad (1)$$

where Φ is the cumulative distribution function of the standardized Normal distribution. The probability of failure can be expressed through a performance function g (also referred to as “limit state function”, see [5], [6]) such that a structure is considered to survive if $g > 0$ and to fail if $g \leq 0$. According to EN 1990, P_f and β are only notional values that do not necessarily represent the actual failure rates. They are used as operational values for code calibration purposes and comparison of reliability levels of structures. For structural elements of Reliability Class RC2 (as defined in EN 1990, Annex B), for the ultimate limit state, the recommended value of β is 3.8.

In general, a limit state function “ g ” as defined above can be formulated for a given structure or structural member, but the probability of this function being smaller than zero or equal to zero, i.e. the probability of failure, is not always easy to be assessed. This is mainly due to the fact that the limit state function in general contains a large number of variables, with different probability distribution functions. Exact analytical integration, numerical

integration, approximate analytical methods and simulation methods are among the most used methods of solving the probability of structural failure. Analytical integration and approximate analytical methods such as First Order Reliability Method (FORM) are limited to simple models. Numerical integration can handle more complex models, but however, the application is limited. Simulation methods have been developed lately along with the development of computers and practically, they can lead to solution for very complex models.

Simulation methods are used in this paper for the reliability analysis of a reinforced concrete bridge pier. Using computer simulations has an important advantage among the other methods; it allows a large number of variables into analysis. The limit state function “ g ” can contain several geometric variables (such as length of elements, dimensions of cross sections, rebar diameters etc.), resistance variables (concrete strength, steel yield strength etc.) and action variables (self-weight of materials, environmental actions and imposed loads). Assessing the probability of a function “ g ” with many independent variables being equal to or smaller than zero would be almost impossible without simulations.

A Monte Carlo simulation is a mathematical technique that involves a (usually) large number of iterations with different random values of inputs, each of which produces a different outcome. Monte Carlo simulations make it possible to study very complex problems and they suit the needs of reliability analysis of structures. Reliability design concepts and techniques are explained further in [5], [6] and [7].

2 RELIABILITY ANALYSIS

2.1 Description of the bridge pier

The transversal section of the bridge is shown in Figure 1. The bridge has several piers in a distance of 20m (span length). For the analysis of the pier, the

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simplified cantilever model shown in Figure 1 was used, with $H=8\text{m}$ and concentrated mass "m".

The study is focused on the seismic design situation. The loads acting on the bridge pier are the axial forces from self-weight and traffic (N), bending moment (M) from eccentricity of traffic loads and the seismic forces. The pier is considered to be adequately fixed at basement. Geotechnical aspects are not considered further in this paper for the reliability analysis of the pier.

The cross section of the bridge pier shown in Figure 1 has nominal dimensions $h=3.4\text{m}$, $b=2.2\text{m}$ and $R=1.1\text{m}$. The ground type according to Eurocode 8 (see [3]) is considered of Type A.

2.2 Definition of variables

When a structure is designed or when an existing structure is assessed, it is impossible to have perfect knowledge for all the factors that influence the results of calculations. Even for the simplest structures, several uncertainties exist. As it is shown in Figure 2, for the bridge pier, it cannot be sure if the cross section has the required dimensions, if reinforcement bars are placed perfectly correct, if their diameter is equal to the specified

diameter or if the shape of the bars matches perfectly with the design. Also, it cannot be sure if concrete strength and yield strength of steel will be higher or lower than those specified. The same uncertainties apply to permanent actions, seismic and traffic loads and even the design model of the reinforced concrete section (e.g. the stress block dimensions).

The pier can be either a new one or an existing one. In the case of a new pier, the Probability Distribution Functions (PDFs) of the variables would represent the probable values. In case of existing bridges, if sufficient tests are available these PDFs will represent the actual distributions.

Recommendations from literature have been used in this paper for the distribution functions of the variables summarized in Table 1. For resistance and loads, the distributions have been chosen with mean value and coefficient of variation such that the characteristic fractile corresponds to a predefined value (see the comments column in Table 1). Model uncertainties have been introduced as multiplicative stochastic variables in the design equations (e.g. multiplying the other variables), with mean value equal to 1. In Table 1 (F) denotes actions, (R) resistances and (a) geometry variables.

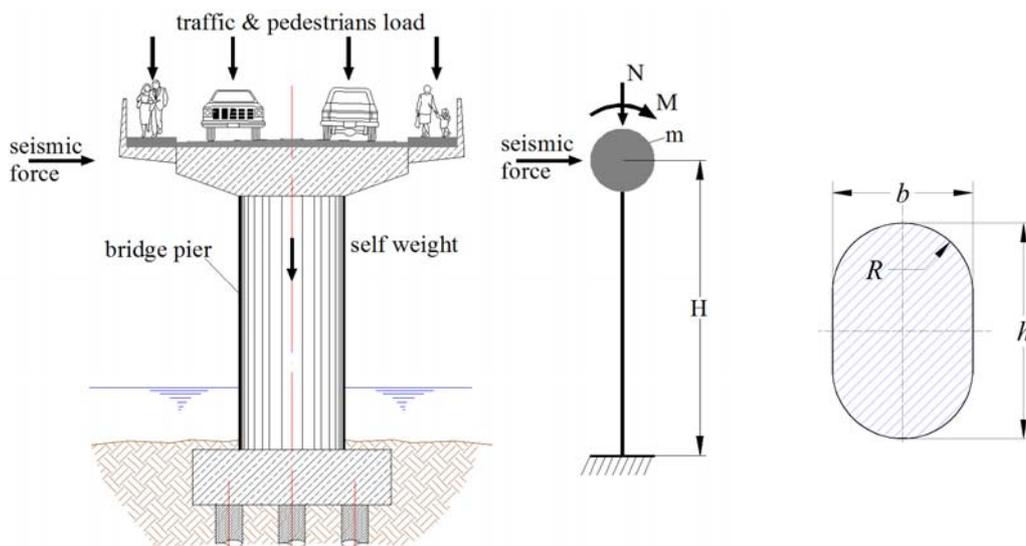


Figure 1. Section of the bridge showing the pier under analysis (left), structural model (middle) and cross section of pier (right)

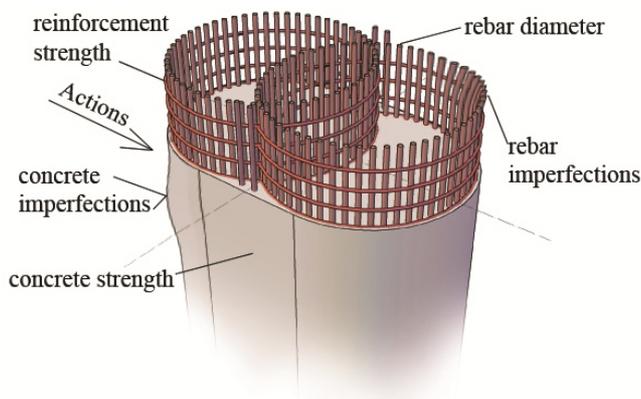


Figure 2. Some of the possible imperfections and uncertainties in the reinforced concrete pier

Table 1. Definition of variables considered

Nr.	Variable	Distribution	Mean	Coef. of variation	Unit	Comment
1	Weight of reinforced concrete (F)	Normal	25	10%	kN/m ³	Based on [9]
2	Traffic loads (F)					LM1 with 5% probability of exceedance in 50 years, see [2], [4]
3	Seismic action (F)					$a_g=0.25g$ with 10% probability of exceedance in 50 years, [3]
4	Concrete compression strength (R)	LogNormal	38	15%	MPa	Characteristic value 30MPa (5% fractile), [1], [5], based on [9]
5	Reinforcement yield strength (R)	LogNormal	430	5%	MPa	Characteristic value 400MPa (5% fractile), [1], [5], based on [9]
6	Model uncertainty	LogNormal	1	5%	-	Multiplicative variable, based on [9]
7	Concrete modulus of elasticity (R)	Normal	3.4×10^7	1%	kN/m ²	Assumption
8	Reinforcement modulus of elasticity (R)	Normal	2.0×10^8	0.6%	kN/m ²	Assumption
9	Height of the pier (a)	Normal	8.0	0.06%	m	Assumption
10	Span length (a)	Normal	17.0	0.06%	m	Assumption
11	Deck section area (a)	Normal	10.22	0.1%	m ²	Assumption
12	Height of pier section (a)	Normal	3.4	0.06%	m	Assumption
13	Pier rebar diameter (a)	Normal	30	0.3%	mm	Assumption
14	Weight of road layers (F)	Normal	28	10%	kN/m ³	Assumption, based on [9]

The seismic hazard for the bridge pier is represented by a reference ground acceleration $a_{gR}=0.25g$ with a probability of exceedance equal to 10% in 50 years. Assuming that the maximum ground acceleration is inaccurately known, a random ground acceleration (larger than 0.25g) multiplied by a Bernoulli variable with mean value 0.1 (as shown in Figure 3) is assumed to represent the "strong" seismic event. For earthquakes with ground acceleration smaller than 0.25g, the bridge pier was analyzed separately.

Considering the seismic design situation, the probability of simultaneous occurrence of maximum values of seismic actions and traffic actions was assumed to be 1%. Further studies are required for a more accurate assessment of the probability of simultaneous occurrence of these two actions.

The stress strain curves for concrete and steel were also considered stochastic, based on the distribution functions of concrete strength and steel yield strength. For a random value of concrete strength and yield strength of reinforcement, the stress strain curves are shown in Figure 4.

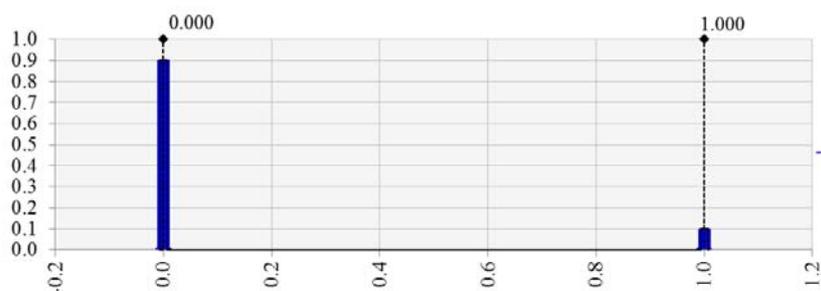


Figure 3. PDF of the multiplicative Bernoulli variable representing the "strong" seismic event

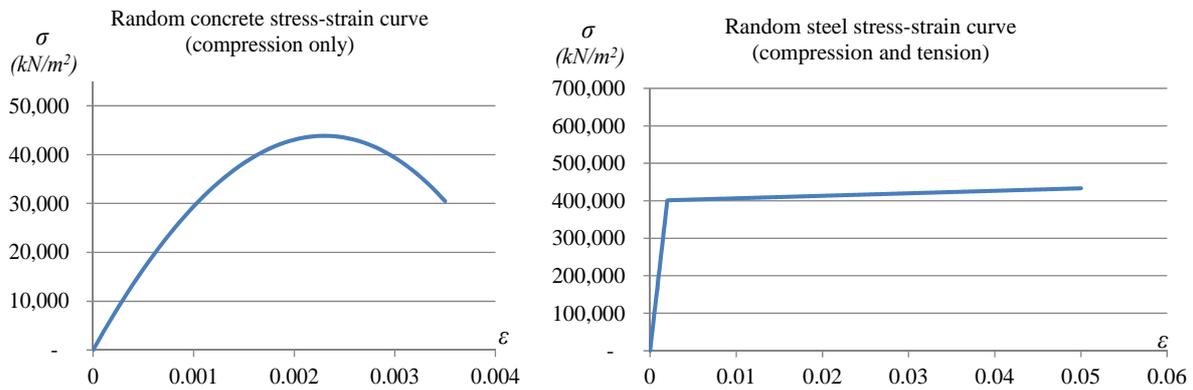


Figure 4. Stress strain curves of concrete and reinforcement for a random sample

In total 14 independent variables were considered for analysis. In general, more detailed and accurate distribution functions can enter the analysis if sufficient information is available (for example, from tests).

2.3 Description of the procedure

A worksheet in Microsoft Excel was built for the analysis of the pier. The basic idea is to run the procedure contained in the worksheet a large number of times while giving random values to the input variables and to collect and analyze the results. In other words, the random process of structural failure or survival has been modelled through a Monte Carlo simulation. In order to perform this simulation, specialized software Palisade @Risk was used. Figure 5 describes the whole procedure using a schematic algorithm.

After the definition of the input variables, the next step is "sampling". For each of the input variables defined in Table 1, samples are taken using the Monte Carlo method, then the internal forces and

displacements are calculated through the worksheet and the results are stored in Microsoft Excel to be further processed. The procedure is repeated until a predefined number of iterations are performed.

The number of iterations performed in a simulation is important. The expected probability of failure is in the range of 1×10^{-4} or less, because for an index of reliability $\beta=3.8$, the probability of failure will be:

$$P_f = \Phi(-\beta) = \Phi(-3.8) = 0.00007235 \quad (2)$$

In other words, if only 10,000 iterations were performed in a simulation, 0 or 1 failure event could have been observed, which means that the possible error is high. In order to reduce the uncertainty of the estimate of probability, several simulations were performed, with number of iterations per simulation ranging from 100,000 to 2,000,000 until a satisfactory convergence was achieved. The estimated probability of failure at the end of the simulation is calculated as:

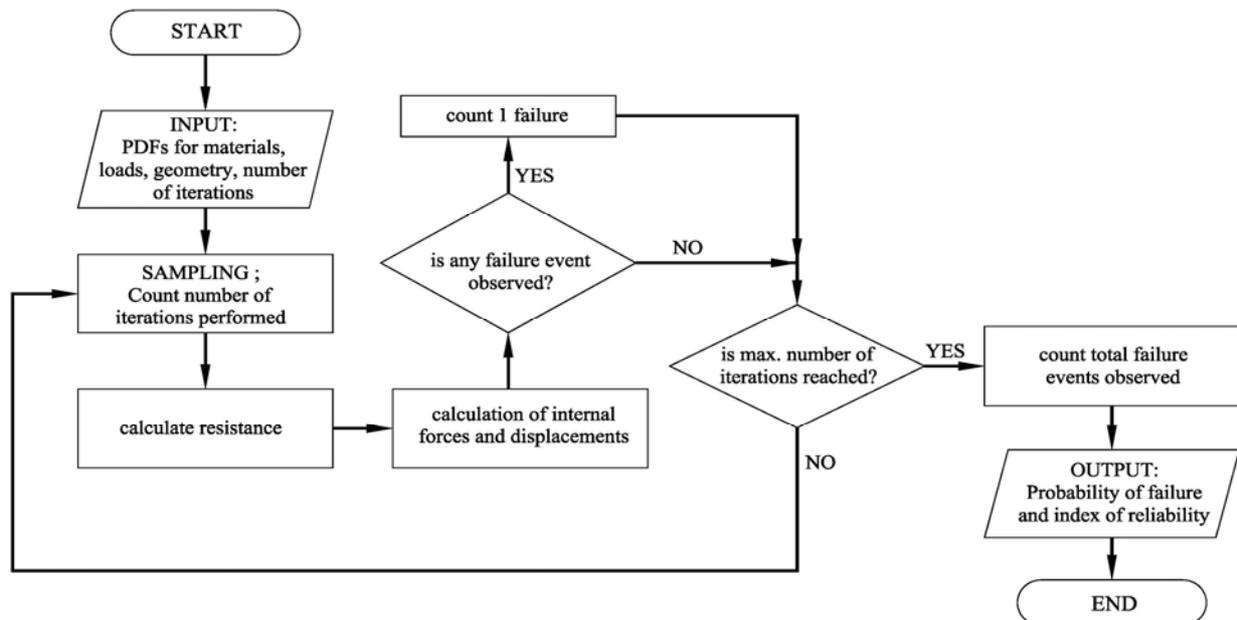


Figure 5. General algorithm for the estimation of index of reliability through Monte Carlo simulation

$$P_f = \frac{\text{number of failures observed}}{\text{number of iterations performed}} \quad (3)$$

2.4 Limit state function

Rather than formulating a limit state function “ g ” as the difference between the resistance and the action effects, a logical function that takes only values “1” and “0” was built (see the algorithm of Figure 5). If failure is observed, the function “ g ” takes value “1”, otherwise its value is “0”. So, the distribution function of “ g ” is a Bernoulli function with its mean value equal to the probability of encountering value “1”, i.e. equal to the probability of failure.

In order to calculate the resisting bending moment and axial force, the cross section was divided into layers as described in Penelis and Kappos [8]. For each iteration, the resisting axial force and the corresponding moment were calculated from the equilibrium of forces resulting from strain profile in Figure 6, with the assumption that plane sections remain plane after deformation.

In Figure 6, x is the depth of neutral axis and ε denotes the strain of concrete (where index “ c ” is used) or steel (with index “ s ”) at any layer i . The following

equilibrium equations shall apply when 20 layers are considered:

$$\begin{cases} N = \sum_{i=1}^{20} \sigma_{ci} A_{ci} + \sum_{i=1}^{20} \sigma_{si} A_{si} \\ M = \sum_{i=1}^{20} \sigma_{ci} A_{ci} y_{ci} + \sum_{i=1}^{20} \sigma_{si} A_{si} y_{si} \end{cases} \quad (4)$$

The stresses σ are calculated from the stress-strain diagrams presented in paragraph 2.2 for a given strain ε . Because of the shape of the pier and the reinforcement layout, the same number of layers was used for both concrete and steel. The equilibrium in (2.3) is fulfilled for a neutral axis depth x which is calculated through iterations (see [8]).

Shear resistance of the pier was excluded in reliability analysis. Further studies can consider shear resistance and all the relevant failure modes, including geotechnical aspects.

Action effects are calculated based on Figure 7.

The seismic force is calculated through the elastic response spectrum for ground Type A as the product of mass “ m ” with the spectral acceleration calculated using Eurocode 8. The mass is calculated for each iteration of the simulation, and it takes into consideration the self-weight of the bridge superstructure and pier and the traffic loads (if present).

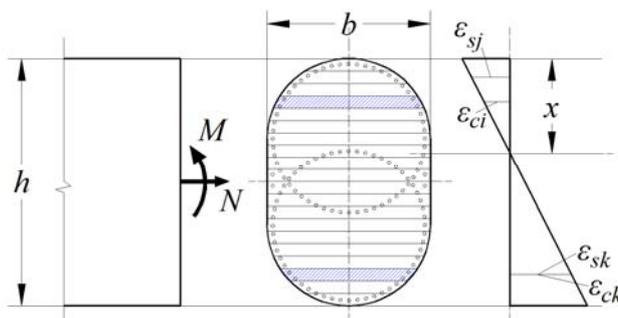


Figure 6. Cross section of the pier and the strain profile

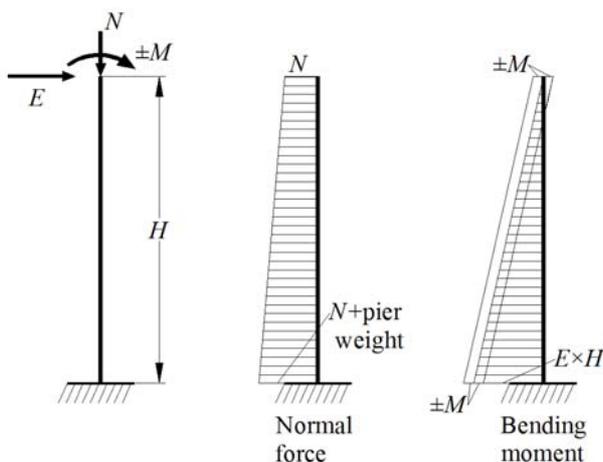


Figure 7. Normal force and bending moment in the bridge pier

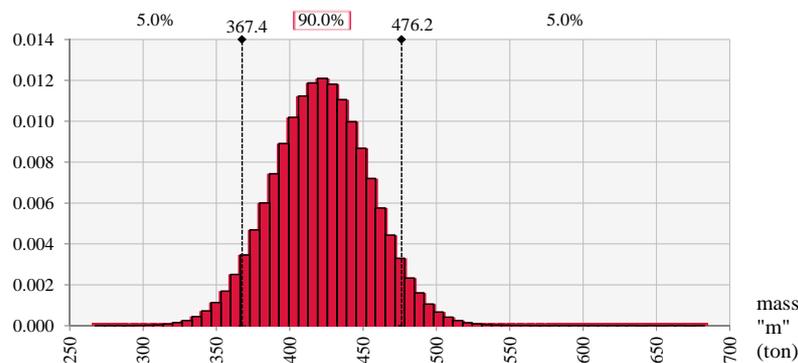


Figure 8. PDF of mass of the deck and pier in tons

Table 2. Estimated probability of failure for different number of iterations performed

Description	Number of iterations in a simulation			
	100,000	500,000	1,000,000	2,000,000
Probability of failure of the pier	0.0000100	0.0000110	0.0000200	0.0000205
Index of reliability β	4.26	4.24	4.11	4.10

No partial factor or combination factor was included in the calculations presented above. The direct comparison of resistances with action effects described above leads to the result of $g=1$ or $g=0$ in an iteration. The procedure then follows the algorithm of Figure 5.

3 RELIABILITY ANALYSIS RESULTS

As described in paragraph 2.2, the ground acceleration during simulation exceeds 0.25g in roughly 10% of iterations. Figure 8 shows the PDF of the concentrated mass “m” (see Figure 1). The variation of mass is due to geometrical variables, self-weight variables and the traffic variables. It is very important because it has direct influence on the fundamental period of the structure and the seismic force.

The analysis results are given in Table 2, for 100000, 500 000, 1000 000 and 2000 000 iterations per simulation. The probability of failure and the accompanying index of reliability are calculated in the spreadsheet for each case and reported in the last row of the table.

Trial simulations with more than 2 million iterations showed that the probabilities estimated have insignificant differences. As a conclusion, based on Table 2, the bridge pier has a reliability level higher than the target value of “3.8”.

4 COMPARISON WITH PARTIAL FACTOR METHOD OF EUROCODES

The already created spreadsheet was used for the design of the reinforced concrete pier according to Partial Factor Method of Eurocodes, considering the seismic design situation. This time, the design values of the variables were used in the design equations, taken from the characteristic value, as defined in the Eurocodes, multiplied or divided by the relevant partial

factors. The traffic loads LM1 were multiplied by the factor $\psi_{2,1}=0.2$ (see [1] to [5]).

In order to make the comparison possible, the design of the reinforced concrete pier according to Eurocodes using partial factors was done prior to the reliability analysis presented in the previous paragraphs. So, the area of reinforcement that resulted from the design according to Eurocodes is the same as the area of steel used for the reliability analysis. A design according to Eurocodes should lead to a reliability index larger than 3.8 (see [1]). The exact value of the index is “invisible” while designing using the partial factors.

On the other hand, the reliability analysis described in this paper leads to an estimation of the index of reliability. In our case, $\beta=4.1$.

5 CONCLUSION

Nowadays, computers offer a great tool for the structural engineer to solve complicated tasks. This paper presented in brief the procedure followed for the assessment of structural reliability of a reinforced concrete bridge pier. It was shown that simulation models can be implemented in calculation spreadsheet in order to solve complicated probability problems related to structural engineering. Given sufficient data is available, it is possible to actually design a structure or to assess its resistance and capacity based on the target reliability level. For the studied pier, the Eurocode Index of Reliability estimated through simulations resulted greater than the target index equal to 3.8. This means that, with the given input data, a more economical design could be possible. Especially for important structures such as bridges, simulation methods can lead to a realistic assessment of structural risk. The index of reliability gives a more clear idea regarding the safety of a structure. Especially for structures being designed with a target index of reliability different from 3.8 (smaller or larger), for which there are no explicitly recommended

partial factors in Eurocode, reliability analysis through simulations can be useful to compare the level of reliability with the target level.

ACKNOWLEDGEMENTS

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SUMMARY

EVALUATION OF STRUCTURAL RELIABILITY USING SIMULATION METHODS

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Eurocode describes the “index of reliability” as a measure of structural reliability, related to the “probability of failure”. This paper is focused on the assessment of this index for a reinforced concrete bridge pier. It is rare to explicitly use reliability concepts for design of structures, but the problems of structural engineering are better known through them. Some of the main methods for the estimation of the probability of failure are the exact analytical integration, numerical integration, approximate analytical methods and simulation methods. Monte Carlo Simulation is used in this paper, because it offers a very good tool for the estimation of probability in multivariate functions. Complicated probability and statistics problems are solved through computer aided simulations of a large number of tests. The procedures of structural reliability assessment for the bridge pier and the comparison with the partial factor method of the Eurocodes have been demonstrated in this paper.

Key words: structural reliability, index of reliability, probability of failure, Monte Carlo simulation, bridge pier

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REZIME

VREDNOVANJE KONSTRUKCIJSKE POUZDANOSTI KORIŠĆENJEM METODA SIMULACIJE

Markel BABALLĚKU
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U Evrokodu 0 opisan je „indeks pouzdanosti“ kao mera konstrukcijske pouzdanosti, koja se odnosi na „verovatnoću otkaza“. U članku naglasak je na procenu pomenutog indeksa za armiranobetonski stub mosta. Nije uobičajeno eksplicitno korišćenje koncepta pouzdanosti u projektovanju konstrukcija, ali se problem konstrukcijskog inženjerstva bolje se razume preko nje. Neke od najvažnijih metoda za procenu verovatnoće otkaza su egzaktna analitička integracija, numerička integracija, aproksimativne analitičke metode i metode simulacije. U ovom radu je korišćena metoda Monte Carlo simulacije, jer nudi veoma dobar alat za procenu verovatnoće u multivarijante funkcija. Komplikovana verovatnoća i statistički problemi su rešeni pomoću komputera koristeći simulacije velikog broja ispitivanja. Procedure procene konstrukcijske pouzdanosti supca most i upoređenje sa metodom parcijalnih faktora Evrokodova su ilustrovane u ovom radu.

Ključne reči: konstrukcijska pouzdanost, indeks pouzdanosti, verovatnoće otkaza, Monte Carlo simulacija, stubac mosta

METODE ODREĐIVANJA POTENCIJALA ZA REVITALIZACIJU GRADITELJSKOG NASLEĐA – Pouke grada Berna

METHODS FOR DETERMINATION OF REVITALIZATION POTENTIAL OF BUILT HERITAGE - Lessons learned on the city of Bern

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1 UVOD

U novoj situaciji, nastaloj poslednjih decenija u Centralnoj i Jugoistočnoj Evropi, kada je reč o graditeljskom nasleđu i njegovoj ulozi u budućnosti grada, korisno je obaviti uporedno proučavanje raznih alternativnih pristupa rešavanju problema. Osnovno pitanje koje se može postaviti jeste kakve su mogućnosti prenosa iskustva iz sredina s tržišnom ekonomijom i na koji način se mogu primeniti u društvu koje je u dugoročnoj, iscrpljujućoj tranziciji, kao što je srpsko društvo.

Evropski stavovi o politici upotrebe nasleđa uglavnom su usmereni ka politici revitalizacije istorijskih centara zbog sledećih razloga:

- U istorijskim gradovima revitalizacija centra je bitna komponenta urbanog menadžmenta. To je „alatka“ za formulisanje politike i implementaciju, te deluje kao ključni faktor za uspeh revitalizacije gradskog centra.

- Mnogi gradovi u Centralnoj i Jugoistočnoj Evropi nemaju jasnu politiku revitalizacije, odnosno regeneracije istorijskih centara. Dramatične promene u ovim delovima Evrope, koje su samo površinski stišane, zahtevaju novi pristup odnosu prema nasleđu i nove instrumente za upravljanje ovim tipom urbanih promena.

- Postepeno propadanje grada i proces njegove regeneracije treba interpretirati u širem kontekstu urbane dinamike. Ovi procesi su veoma nejasni i rasplnuti. Uporedna analiza može pomoći da se brže dođe do objašnjenja, koristeći iskustva onih zemalja u kojima su pojedini problemi uspešno savladani.

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1 INTRODUCTION

In the new circumstances created in the Central and Southeast Europe in the recent decades, when the built heritage and its role in the future of the cities are concerned, it is useful to undertake a comparative study of various alternative approaches to the problem solution. The fundamental question which can be posed is what is the potential for transferring the experiences from the environments with market economies, and in what way they can be implemented in a society in a long-running, exhausting transition, such as the Serbian one.

The European position on the policy of heritage treatment is mostly directed towards the policy of revitalization of historical centres because:

- In the historical cities, revitalization of the centre is an important component of urban management. It is a “tool” for policy formulation and acts implementation as a key factor for success of city centre revitalization.

- Many cities in Central and Southeast Europe have unclear revitalization policy, i.e. regeneration policy of historical centres. Dramatic changes in these parts of Europe, which only superficially abated, demand a new approach in the attitude towards the heritage and new instruments for management of this type of urban changes.

- Gradual deterioration of the city and the process of its regeneration should be interpreted in a wider context of urban dynamics. These processes are very vague and divergent. A comparative analysis can help to

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- povećanje konkurentnosti istorijskog jezgra kao investicione lokacije; privlačenje novih „unutrašnjih“ ulaganja putem pojačanog marketinga istorijskog jezgra i upotrebom raznih logističkih podršaka	- podrška odnosno pomoć postojećem lokalnom biznisu i zaštita stanovnika s malim primanjima; izbegavanje izmeštanja stanovnika; promovisanje svesti o zajedništvu i identitetu, kao i osnivanje pokreta „samopomoći“
- modernizacija urbane strukture i promovisanje marketinških smernica za nove arhitektonske forme koje su značajno slobodnije, fleksibilnije i odgovarajuće tržištu	- zaštita i očuvanje starih struktura i stilova kroz strogu kontrolu prostornih promena i estetskih standarda
- stvaranje novih i snažnih vizija o istorijskom jezgru grada, kao simbolu ekonomske i političke moći korišćenjem novog jezika urbanih formi i stvaranjem nove skale vrednosti	- čuvanje prostorne slike istorijskog jezgra kao simbola duhovnih i kulturnih vrednosti; čuvanje postojećih urbanih odnosa i semiotike gradske slike

Increase of competitiveness of historical core as an investment location; attraction of new “internal” investment using intensified marketing of the historical core and using diverse logistical support.	Support, i.e. aid to the existing local business and protection of population with low income; avoiding population displacement; promotion of community awareness and identity and founding of the “self-support” movement
Modernization of the urbane structure and promotion of marketing guidelines for new architectonic forms which are considerably freer, more flexible and more appropriate for the market.	Protection and preservation of old structures and styles through the strict control of spatial changes and aesthetic standards.
Creation of new and powerful visions on the historical core of the city, as a symbol of economic and political power, using new language of urban forms, and creating a new scale of values	Preservation of the spatial image of a historical core as a symbol of spiritual and cultural values; preservation of existing urban relationships and semantics of urban image

Nakon uočavanja razlika, može se postaviti pitanje da li se ekonomski razvoj i kulturne vrednosti uzajamno isključuju. Niz veoma uspešno rešenih primera revitalizacije istorijskih gradskih jezgara u Evropi, pa i u Americi, pokazuje da se ovi konflikti mogu ublažiti, pa čak i potrti. Urbana politika je umetnost kompromisa. Zato se regeneracija gradskog jezgra definiše kao niz programa i planiranih akcija, osmišljenih tako da učine održivim i unaprede postojeće urbano tkivo, ali i da uvedu nove aktivnosti koje mogu da unaprede ekonomsku bazu grada.

U međunarodnom iskustvu ovaj pristup naziva se *kulturno održiva ekonomska revitalizacija istorijskog gradskog jezgra* [14]. U izvesnom smislu, ovaj termin nastao je pod uticajem ideje o održivom razvoju, ali pored toga, sadrži u sebi i prihvata još neke pristupe kao što su: istorijska zaštita, integrativna konzervacija; „meka ili oprezna obnova“; koncept „zdravog grada“; stavovi pokreta „stvaranje životnog grada“, i druge. Osnovna ideja jeste da se pozitivno posreduje u konfliktu između ekonomskih interesa, zaštite nasleđa i unapređivanja okruženja. Kao primer veoma povoljno primenjene *kulturno održive ekonomske revitalizacije istorijskog gradskog jezgra*, izdvaja se primer grada Berna u Švajcarskoj, koji će u ovom radu biti analitički obrađen, s namerom da se izvuku određene pouke pogodne za rešavanje brojnih problema u gradovima Srbije.

essence of spatial policy. This general observation gains a special meaning when planning in historical cities is concerned, where the fundamental urban conflicts between the economic development and cultural values are undoubtedly most dramatic.

After observing differences, one may pose a question whether economic development and cultural values are mutually exclusive? A number of successfully solved examples of revitalization historical city cores in Europe and America indicate that these conflicts can be attenuated, and even annulled. Urban policy is the art of compromise. For that reason regeneration of city core is defined as number of programmes and planned actions conceived so as to make the existing city fabric sustainable and improve it, but also to introduce new activities which can improve the economic basis of the city.

In the international practice, this approach is called “culturally sustainable economic revitalization of the city core” [14]. In a sense, this term was created under the influence of the idea of sustainable development, but contains and endorses some other approaches such as: historical protection, integrative conservation; “soft or careful renovation”; “healthy city” concept; positions of the movement “creation of a vital city”, and others. The basic idea is to be positive intermediary in the conflict between the economic interests, protection of heritage and improvement of the environment. The example of the city of Bern, Switzerland, stands out as an example of a very favourably implemented “culturally sustainable economic revitalization of the city core”, and it will be analytically treated in this paper, with the intention to draw certain lessons suitable for solving problems which piled up in the cities of Serbia.

2 PRISTUP PROCESU REVITALIZACIJE

Zaštićene zgrade su osnovni oblik našeg kulturnog pejzaža. Oni su vodilja u razvoju jednog mesta u kome se nalaze u smislu „korporativne arhitekture”, odnosno kako pojedinačnog, tako i zajedničkog interesa. Korist od „korporativne arhitekture” samih objekata ima, pre svega, kompanija koja je taj objekat obnovila, odnosno koja ga je obnovljenog stavila u funkciju. Budući korisnici, međutim, trebalo bi da budu svesni vrednosti istorijskog i ekonomskog karaktera takvih zaštićenih objekata.

U isto vreme, postoje mnogi istorijski zaštićeni objekti kojima preči propadanje i rušenje usled nekorišćenja. Oživljavanjem takvih objekata može biti spašen ne samo izuzetan objekat, već se može oživeti i cela gradska sredina u kojoj se taj objekat, odnosno ti objekti nalaze. No, to zahteva primenu novih ideja i metoda za procenu upotrebljivosti objekata kada je reč o potencijalnim korisnicima.

Karakteristika strukturâ praznih spomenika kulture jeste da su smešteni u postojećim „luksuzima praznine”. Pravi primeri za buduće korišćenje ovih zaštićenih objekata jesu oni objekti koji su revitalizovani i na kojima su sve potrebne mere kompenzacije izvedene. Mere kompenzacije su procenjene prema podobnosti zaštićenog spomenika kulture koji se revitalizuje.

Ovaj rad zasnovan je na polazištu da su u zaštićenim spomenicima kulture utemeljene i imanentne poruke jedne epohe. Istovremeno, analizirajući istorijske objekte i zgrade u vezi sa ovim procesom, mogu se identifikovati potencijali njihovih potreba i njihovog očuvanja.

Nakon analize ovog procesa, koji se odnosi na potrebu očuvanja istorijskih objekata i zgrada, sledeći postupak jeste usmereni pristup zaštićenim objektima.

Istraživački proces koristi odgovarajuće analize i kreativne metode za postizanje održivog razvoja, odnosno održive koristi, što ima za posledicu regeneraciju zaštićenih objekata.

Zato je osnovni cilj ove metode da se otkriju potencijali revitalizacije da bi se povećala konkurentnost objekta u urbanom tkivu.

Pristup revitalizaciji zaštićenih spomenika kulture, u svrhu budućeg korišćenja, može se podeliti u četiri modula.

Modul 1 Uključenje zgrade u savremeni život na osnovu korišćenja istorijskih procesa i njenih samih karakterističnih parametara. Nove ideje se generišu i koriste u sledećem modulu.

Modul 2 Utvrđivanje koristi od istraživačkih kreativnih radionica, koje ocenjuju budući korisnici.

Modul 3 Upoređivanje zahteva za budući život, u pogledu upotrebe i funkcije postojeće zgrade. Kao rezultat toga, proizlaze mere kompenzacije kojima se raspolaže.

Modul 4 Ispitivanje podobnosti očuvanja objekta iz perspektive korisnika. Uspešnost opisane metodologije proverena je na primerima analiziranim u gradu Bernu. Na osnovu analiziranog procesa primene metode, zaključeno je da se ona može, uz prilagođavanje našem zakonodavstvu, primenjivati u našoj sredini.

2 APPROACH TO REVITALIZATION PROCESS

Listed buildings are the basic form of our cultural landscape. They are guiding principles in development of a place where they are situated in a capacity of “corporative architecture”, i.e. as both individual and common interest. The “corporative architecture” of the buildings themselves is useful primarily for companies which renovated the building or put it working order after renovation. The future users should, however be aware of the historic and economic value of such protected buildings.

Simultaneously, there are a large number of historically protected buildings which are in danger of dilapidation and collapse due to the disuse. Revitalization of such structures, may not only salvage a remarkable building, but can revitalize the entire city neighbourhood where such building or buildings are situated. This calls for application of new ideas and methods for evaluation of usability of structures for the potential users.

The characteristic of the structures of abandoned (empty) cultural monuments is that they are situated in the existing “luxury voids”. True examples for the future usage of protected buildings are those buildings which were revitalized on which all the compensation methods were performed. The compensation methods are assessed according to the suitability of the protected cultural monument being revitalized.

This paper is based on the viewpoint that the listed cultural monuments embody immanent messages of an epoch. By analysing historical buildings related to this process, the potential of their needs and their preservation can be identified.

After analysing this process, which refers to the need for preservation of historical buildings and structures, the following procedure is the process of directed approach to the protected structures.

The research process utilises adequate analyses and creative methods for achieving sustainable development, i.e., sustainable usefulness, which results in regeneration of the protected buildings.

For that reason the basic goal of this method is to discover the potential for revitalization in order to increase competitiveness of the structures in the urban fabric.

The approach to the revitalization process of protected cultural monuments for the purpose of the future use can be divided into four modules.

Module 1 Inclusion of the building into contemporary life, based on the utilisation of historical processes and characteristic parameters. New ideas are generated and used in the next module.

Module 2 Determination of usefulness of research creative workshops, as assessed by the future users.

Module 3 Comparison of the demands for the future life, which are related to the use and function of the existing building. The result is the compensation measures available.

Module 4 Testing the structure of preservation suitability from the users’ perspective.

The capability of the described methodology was verified on the examples analysed in the city of Bern. Based on the analysed process of method application, it is concluded that it can be implemented in our practice after being adapted to our legislation.

Komparativna analiza i sinteza služe za izvođenje konačnih rezultata. Kriterijumi izbora svode broj objekata na najvrednije i najreprezentativnije predstavnike Fonda kulturne baštine, za koje se definišu rešenja i izvode preporuke za primenjivost i standardizaciju tih rešenja u Srbiji. Sama metodologija predstavlja praktično sredstvo za „analizu korišćenja” u slučaju upotrebe praznih istorijskih zgrada u kreativnom smislu.

Način sprovođenja analize potencijala postojećih objekata jeste takav da se dobijaju rezultati koji su optimalni za primenu neophodnih kompenzacionih mera. Ove mere treba da budu kontrolisane tako što se utvrđuje korist koju one donose budućim korisnicima, te se na osnovu njih daju predlozi za preventivnu zaštitu. Primenom ove analize korišćenja, budući korisnici i investitori dobijaju mogućnost da provere rezultate podobnosti i dobijaju instrument pogodan za dovođenje navedenih nekretnina na željeni, upotrebnii nivo.

2.3 Definisane primenjenih principa, terminologije i metoda rada

Na temu revitalizacije, kada su u pitanju prazni istorijski objekti, u literaturi se mogu pronaći uglavnom osnovne informacije. Praktično dejstvo revitalizacije iskazuje se u delu izrade projekta. Koncept je prvenstveno namenjen za revitalizaciju unutrašnjih delova onih delova zgrade koji su prazni odnosno napušteni. Zadovoljavanje uslova korisnika i investitora zaštićenih spomenika kulture samo je početna tačka u postavljanju ciljeva. Korišćenje i održivost objekata je pitanje o kojem se u oblasti zaštite i očuvanja graditeljskog nasleđa nije puno diskutovalo.[9]

Većina savremenih zakona o zaštiti nasleđa sadrži zahteve za „održivu zaštitu” i upotrebu, ali sam postupak nije precizno definisan. Moderna interpretacija zaštite istorijskih objekata, za koju se u ovom radu zalaže, zasnovana je na otvorenosti upotrebe spomenika kulture. Proces orijentisanog pristupa, koji se temelji na polazištima ovog rada, nalazi svoje osnove u ponovnom upravljanju objektom (*Facility Management*). Kreativne metode su na veoma jasan način opisane u brojnoj literaturi. Postoji, međutim, neslaganje u određivanju i postizanju ciljeva svake metode pojedinačno.

Sistematizacija zahteva, odnosno tipa novih potreba, koje se razlikuju od izvornih, ne postoji. Bilo bi veoma efikasno kada bi se na osnovu želja potencijalnih korisnika mogli odmah praviti i odgovarajući projekti. Jedinствenu klasifikaciju, međutim, komplikuju specifični zahtevi budućih korisnika, kao i njihova neodlučnost i potreba da se stalno unose promene u zahteve. Izuzetak su jedino slučajevi kada su u pitanju Švajcarske državne norme (SIA).[17]

Test i metode za ispitivanje korišćenja dobrih i sigurnih spomenika kulture mogu se primeniti na taj način. Za opis kompenzacionih mera nudi se struktura izražena prema Normi SIA 480, „troškovi u građevinarstvu”. [18]

Predmet istraživanja su zaštićene slobodne, odnosno nenaseljene zgrade koje su spomenici kulture. Fokusanje na zaštićene objekte učinjeno je iz dva

formulated criteria and parameters.

Comparative analysis and synthesis are used for production of the final results. The Selection criteria bring down the number of buildings to the most valuable and most representative examples of the Cultural Heritage Fund, for which the solutions are defined and recommendations are made for feasibility and standardization of such solutions in Serbia. The methodology itself represents a practical instrument for “utilization analysis” in case when abandoned historical buildings are used in a creative manner.

The method of conducting the analysis of potential of existing structures is such that the results optimal for implementation of necessary compensation measures are obtained. These measures should be controlled by determining the benefit for their future users and on whose basis the proposals for preventive protection are given. By implementing this utilization analysis, the future users and investors obtain the possibility to check the suitability results and obtain an instrument convenient for bringing the mentioned real property to a desirable, usability level.

2.3 Definition of applied principles, terminology and working method

Regarding revitalization, when it comes to the abandoned historical buildings only basic information can be found in references. Practical effects of revitalization are manifest in the design production phase. The concept is primarily intended for revitalization of internal parts of those parts of the building which are empty, i.e. abandoned. Meeting the conditions of users and investors of the listed cultural monuments is only an initial point in goal setting. Utilization and sustainability of structures is an issue which has never been extensively discussed in the field of protection and preservation of built heritage. [9]

Majority of contemporary regulations on the heritage protection contains the demands for “sustainable protection” and usage, but the procedure itself is not precisely defined. Contemporary interpretation of historical building protection, which is advocated in this paper, is based on the openness of the cultural monument to usage. The process of oriented approach of this paper is based on the reintroduced *Facility Management*. The creative methods are in a very clear way described in the literature. There is, however, disagreement in setting and achievement of the goals of each individual method.

Systematization of requirements, i.e. type of new demands which differ from the original ones, does not exist. It would be very efficient if adequate designs were immediately created based on the wishes of potential users. A uniform classification, however, is complicated by the specific requirements of the future users, as well as their indecisiveness and need to continually alter the requirements. The exceptions are only those cases when *Swiss state standards* are concerned, (SIA). [17]

The test and testing methods for good and safe cultural monuments can be implemented in this way. For description of compensation measures, the structure expressed according to Norm SIA 480 is offered, „Costs in civil engineering”. [18]

The subject of the research are listed vacant, that is,

mogu se uočiti dve alternative, definisane na sledeći način:

- Alternativa 1: Poređenje svojstva zgrade sa zahtevima raznih budućih korisnika da bi se izbegle neprikladne namene korišćenjem regulativnih kriterijuma, a istovremeno identifikovao i ograničio broj pogodnih (odgovarajućih) namena.

Prednost ove alternative je to što proces može biti razvijan kao instrument za ocenjivanje, zasnovan na naučnoj osnovi; mana je to što su moguće namene već zadate. U tom slučaju, izostaje kreativni momenat, a i osobenosti pojedinačnih objekata su smanjene i ne mogu se uzimati u obzir.

- Alternativa 2: U okviru neke ustanove, biroa ili studija ostvaruje se uspešan odabir potencijalnog korisnika, putem specifičnog, kreativnog procesa, problemki postavljenog.

Prednost ove alternative jeste činjenica da je moguće pronaći nova i netradicionalna rešenja koja su uspešna. Potencijal objekta sagledava se putem procesa koji se kasnije može iskoristiti za marketing. Mana ovog metoda je to što nije naučno zasnovan. Rezultati zavise od dobre pripreme i kvalitetno urađenog posla arhitekata i drugih zaposlenih.

Upoređivanjem prednosti i mana navedenih alternativa, može se zaključiti da Alternativa 2 pruža više mogućnosti za uspešan praktični rad. Kreativne ideje se dalje preispituju i proveravaju.

2) Predlog za akciju i opis metodologije

Objekat je ispitivan imajući u vidu mogućnost objektivnog adekvatnog korišćenja, bez saznanja da li je implementacija korišćenja nekog konkretnog korisnika moguća. Ovakvo ispitivanje omogućava neku vrstu standardne ponude za različite namene, bez obzira na korisnika.

Pošto ne postoji popis zahteva za različite upotrebe, stvara se opšta lista – s jedne strane, parametara zgrade; s druge strane, zahteva korisnika za procenu uspešnog korišćenja uz održivost zaštite. Obim procene ispitivan je radi ocenjivanja praktičnosti i tačnosti dobijenih rezultata. Konačni rezultat dobijen je u vidu jedne vrste agende s listom mogućih namena, koja stoji na raspolaganju svima zainteresovanim.

Na primeru praznih, nenaseljenih, a zaštićenih zgrada, koje su bile u vlasništvu Kantona Bern, Menadžment i Direkcija za zaštitu objekata grada Berna ispitali su ovu razvojnu metodu i primenili je u praksi na način prikazan u Tabeli 2.

Učešće švajcarske države u ukupnom finansiranju UNESCO-a izraženo je u procentima ekonomske moći, a iznosi 1,22% ili, pretvoreno u brojke – oko 5,2 miliona švajcarskih franaka. Švajcarska je na četrnaestom mestu po finansiranju UNESCO-a. Imajući u vidu veličinu države i broj stanovnika, iznos koji je Švajcarska izdvojila veoma je velik. Ovoliko izdvajanje, kao ni znatno manje, Republika Srbija ne može da realizuje, što može otežati primenu modela zaštite i revitalizacije spomenika kulture na listi Svetske baštine u Srbiji.

future user of the building. In the course of method formation, two alternatives can be noticed, and they are defined in the following way:

- Alternative 1: Comparison of the building properties with the requirements of various future users in order to avoid the improper uses applying regulatory criteria, and to simultaneously avoid and identify the number of suitable (appropriate) uses.

The advantage of this alternative is that the process can be developed as an instrument for assessment, based on the scientific grounds. The drawback is that the potential use is already set. In this case, there is no creativity, and the characteristics of individual buildings are reduced, and cannot be taken into consideration.

- Alternative 2: In an institution or bureau the successful selection of a potential user is performed, through a specific problem oriented creative process.

The advantage of this alternative is the fact that it is possible to find new and non-traditional solutions which are successful. The potential of the building is viewed through the process which later can be used in marketing process. The disadvantage of this method is that it is not scientifically based. The results depend on the good implementation and the quality performance of the architects and other participants...

By comparing the advantages and disadvantages of these two mentioned alternatives, it can be concluded that the Alternative 2 provides more potential for successful practical work. The creative ideas are further questioned and verified.

2) Action proposal and methodology description

The building is tested for the potential of objectively adequate use, with no knowledge whether the implementation of the use of some concrete user is possible. Such testing enables some kind of a standard offer for different uses, irrespective of the uses.

Since there is no list of requirements for different uses, a general list is generated: on one hand the building parameters are listed, and on the other, the requirements of the users for the assessment of successful use with sustainable protection. The scope of the assessment was tested for the purpose of appraising the practicality and accuracy of the obtained results. The end result obtained was in the form of an agenda with the list of possible uses, which is at disposal of all the interested parties.

The Management and authorities for protection of the buildings in the city of Bern tested this development method and put it in practice in the way displayed in the Table 2, on the example of abandoned, uninhabited, and listed buildings which were owned by the Bern canton.

The share of the Swiss state in the total financing of UNESCO is expressed in the percentage of economic power, and it amounts to 1, 22% or, converted to numbers, around 5, 2 million of Swiss Francs. Switzerland is ranked 14th in terms of financing. The Swiss contribution of 5.2 million Swiss francs is very high for a state of its size and number of inhabitants. The Republic of Serbia cannot realize such a high (nor even considerably lower) contribution, which may complicate implementation of the protection and revitalization model of the World Heritage monuments in Serbia.

Tabela 2. Prikaz razvojne metode za nenaseljene zaštićene zgrade
 Table 2. Development method for inhabited protected houses

	SADRŽAJ / CONTENTS	METODA / METHOD
Analitičko-deskriptivni deo Analytical descriptive part	Pregled između istraživačkih projekata i menadžmenta zaštite Overview of research projects and protection management	Izazovi rada i problematika Working challenges and problems
	↓	
	Fokusiranje na prazne zaštićene objekte (imobilije) Focusing on abandoned listed buildings	Specificiranje tema Specification of topics
	↓	
Predlog rada i dogovora/Opis metodologije Work and agreement proposal/description of methodology	Izvodljivost i metodologija iskorišćenosti Feasibility and degree use methodology	Istraživanje i pregled različitih kreativnih metoda Research and overview of various creative methods
	↓	
	Izvodljivost metodologije za ispitivanje održivog korištenja Feasibility of methodology for testing of sustainable use	Pregled i kategorizacija zahteva za potražnju i iskorištenost Overview and categorization of requirements for demand and degree of use
	↓	
	Izvodljivost i neophodne kompenzacione mere Feasibility and necessary compensation measures	Pregled i kategorizacija neophodnih kompenzacionih mera Overview and categorization of necessary compensation measures.
	Ispitivanje zaštitnih mera Testing protective measures	Pregled različitih metoda za testiranje Overview of various testing methods
Primer Example	Ispitivanje, provera metoda i provođenje u praksi Testing and verification of methods and practical implementation	Pregled primera u praksi Overview of examples in practice
Rezultat Result	Rezultati i zaključci Results and conclusions	Kvalitetni pregled rezultata i pogled na dalja istraživanja Quality overview of results and perspective of future research

2.4 O staroj gradnji i njenoj zaštiti u Bernu

Objekti koji su zaštićeni kao spomenici kulture dokazi su društvenog života prošlih vremena. Oni su deo kulture jednog društva i svedoče o načinu života stanovništva na jednom području.

Razumevanje objekta zavisi od što tačnijeg poznavanja vremena kada je izgrađen i svih promena koje su na njemu nastale tokom upotrebe. Prikupljene informacije o istorijskom životu zgrade i njene okoline omogućavaju uspešnu sveobuhvatnu obnovu, rekonstrukciju i buduće korišćenje koje će produžiti život zgrade kao spomenika kulture.

Svi prikupljeni podaci, zajedno s tehničkom dokumentacijom i fotodokumentacijom, služe kao podloga za formiranje smernica u pogledu toga kako sa zgradom postupati u budućnosti, koje su intervencije moguće, a koje nisu, te kakve su mogućnosti za promenu namene, ukoliko je to rešenje da se građevina sačuva i uključi u savremeni život grada. Na taj način, stvara se baza podataka, odnosno inventar kulturnih dobara koji služba zaštite spomenika kulture i kancelarija kantona koriste za razne svrhe. S obzirom na broj i preciznost prikupljenih podataka, na osnovu tog inventara moguće je definisati celovit plan delovanja, kao i finansijsku strukturu troškova za svaku potencijalnu, pojedinačnu intervenciju, kao što su konzervacija, sanacija, prenamena funkcije i revitalizacija kao oblik zaštite građevine u njegovoj fizičkoj i duhovnoj celovitosti.

Kada je u pitanju revitalizacija, postoji veoma čvrsto utemeljen uslov da – bez obzira na druge intervencije – spoljni izgled objekta mora da zadrži postojeći izgled. Nikakve intervencije na fasadama u drugim materijalima nisu dozvoljene, osim s kamenom, i to boje koja je karakteristična za Stari grad Bern. Što se tiče izmena u unutrašnjosti objekata, to je regulisano Pravilnikom o zaštiti, tj. nova namena nekog objekta može se ostvariti samo ukoliko se sprovedu sve mere savremene zaštite, uz poštovanje koncepta budućeg korisnika. Zato je u interesu gradske vlasti da, ukoliko je potrebno izvesti prenamenu prostorija ili celog zaštićenog objekta s ciljem njegovog opstanka, u što kraćem periodu pronađe novog korisnika i u saradnji s njim odredi buduću namenu. Ovaj postupak podrazumeva to da se nova namena, koja će se uvesti u objekat, nalazi u registru delatnosti za koje je prethodno utvrđeno da su adekvatne prostoru i mestu u istorijskom, zaštićenom starom gradu. U zavisnosti od vrste nove funkcije, moguće je da su potrebne samo preventivne, jednostavne popravke manjeg obima i one se veoma uspešno obavljaju. Kada nova namena zahteva značajnije promene u unutrašnjem tkivu građevine, onda su često potrebna i nova, savremena rešenja koja će omogućiti revitalizaciju enterijera. Primenjeni materijali za buduće intervencije na objektima ne smeju biti štetni i zagađujući za ljude i okolinu. Obavezan zahtev je da se ovakve intervencije i eventualno dodatno građenje mora obaviti primenom konvencionalnog načina gradnje i ekološki orijentisane konstrukcije.

U tom slučaju, zahtevi novih korisnika već su na neki način unapred sagledani u inventaru mogućih namena, te nailaze na spremnu i ekspeditivnu gradsku službu po svim pitanjima zaštite i revitalizacije.

2.4 On the old building stock and its protection in Bern

The structures which are listed as cultural monuments are evidence of social life of past times. They are a part of the culture of one society and testify the lifestyle of population in certain area.

Building comprehension depends on the accurate knowledge about the time when it was built and the changes it experienced during the long service. The collected information about the historical life of a building and its environment allow a successful comprehensive renovation, reconstruction and future usage which will extend the life of the building as a cultural monument.

All the collected data, along with the technical and photo documents serve as a basis for formation of guidelines of how to treat the building in the future, for finding which interventions are possible, and which are not, and what the potential for function conversion is, if the solution comprises preservation of the building and inclusion in contemporary life of the city. Data basis is created in this way, i.e. inventory of cultural assets which are used by the Cultural monuments protection service and the canton Chancellery office for various purposes. Regarding the number and accuracy of collected data, on the basis of the Inventory, it is possible to define an integral action plan and a financial structure of the costs for each potential, individual intervention such as conservation, rehabilitation, function conversion and revitalization as a form of protection of a building in its physical and spiritual entirety.

When the revitalization is concerned there is a firmly founded condition that, regardless of other interventions, the external appearance of the building should retain the existing appearance. No interventions using different materials on the facades are allowed except with stone, in the colour characteristic of the Old city of Bern. As for the changes inside the buildings, it is regulated by the *Code on protection*, i.e. new function of a structure can be realized only if all the measures of the contemporary protection are conducted with observation of the concept of the future users. Therefore, it is in the interest of the city authorities to convert the rooms or entire listed building if necessary, so it may survive, and to find a new user as soon as possible as well as to determine the future use in cooperation with him. This procedure comprises the new use which will the building in the register be introduced in of the activities which were previously approved to be adequate for the space and location in the historical, protected old town. Depending on the type of the new function, it is possible that only preventive, simple non-extensive repairs are required, and they are very successfully performed. When a new use calls for considerable repairs in the interior fabric of the building, then frequently, new contemporary solutions are required which will enable revitalization of the interior. The implemented materials in future intervention on the structures should be harmless and unpolluted. The basic requirement is that such interventions and potential building extensions should be performed applying conventional method of construction and environmentally oriented structures.

In this case, requirements of new users are, in a way, anticipated through the Inventory of potential uses, and the city service is ready and efficient for all the issues of protection and revitalization.

nasleđa pre Drugog svetskog rata, ali navedene mere, uvedene pre sedam do osam decenija, tu situaciju su značajno promenile. Ta promena se manifestuje, pre svega, u postupnom, ali stalnom posmatranju spomenika, te blagovremenim popravkama, kako se ti propusti ne bi ponovili.

2.6 Inventarisanje spomenika kulture kao preventivna zaštita

Veoma dugo su u Bernu samo neke javne i sakralne građevine bile označene kao spomenici kulture. Na primer, Gradska palata, jedna od najstarijih palata istorijskog dela Berna, od vlasnika i u političkim krugovima označena kao bitan objekat u memoriji grada Berna, kao i Katedrala (*Münster*), započeta 1421. godine, najviša u Švajcarskoj. Broj spomenika kulture se pažljivo i postepeno proširivao.

S velikom pažnjom i brigom, Komisija za održavanje spomenika kulture priprema i sastavlja izveštaje tako da budu razumljivi i ljudima koji nisu stručnjaci, da bi svi bili upoznati s radovima koji će se izvoditi na spomenicima kulture. Ipak, ti spiskovi nisu uvek ispunjavali sve uslove i zato se poslednjih dvadesetak godina intenzivno radi na vrednovanju graditeljskog nasleđa da bi ono moglo da se što kvalitetnije inventariše. Istovremeno, ovaj inventar se digitalizuje u centralnoj bazi podataka, u kojoj se, prilikom popisa, ispunjavaju svi državni, kantonalni i gradski zakoni zaštite objekata. Od početka rada, Komisija se veoma trudila da ono što radi naiđe na razumevanje i prihvatanje građana Berna, jer upravo građani putem raznih poreskih oblika plaćaju rad Komisije. Pre nekoliko godina, ustanove građanske inicijative uspele su da se izbore za pravo odlučivanja i uticaja na odlučivanje prilikom upisa zaštićenih zgrada u spisak Inventara.

Oblici građanske inicijative dobili su „zeleno svetlo“ u ministarstvima Kantona Bern i pravo odlučivanja stanovnika grada podignuta su na viši nivo. Sistem u praksi funkcioniše tako što Komisija popisuje objekte, klasifikuje ih, određuje kategoriju zaštite, ali ukoliko, na osnovu stručnih istraživanja, Komisija odluči da se spomenik kulture mora srušiti ili značajno renovirati, bez obzira na to što će to skupo koštati poreske obveznike, tada građani istupaju sa svojom inicijativom. Građanska inicijativa je mehanizam koji se u Švajcarskoj, pa tako i u Bernu, maksimalno koristi. Čim građani konstatuju da treba da reaguju, oni prikupljaju potpise i raspisuje se referendum o tom problemu, o tome da li treba da se prihvati odluka Komisije ili ne.

Rezultati referenduma obavezujući su za sve učesnike tog procesa. S tim mehanizmom odlučivanja, procesi se odvijaju na obostranu korist. Na taj način, procenjuje se, odnosno prihvata ili kritikuje rad Komisije. Komisija za zaštitu objekata uvek je pod lupom javnosti, njen rad je potpuno transparentan. Javnost i transparentnost glavna su obeležja u radu ove institucije. Osnovni pristup u radu jeste da se problemi rešavaju na

reports of performed controls should be timely submitted and nothing neglected. The services competent for that sort of the work should be informed in detail about how the repairs or protection will be performed, what time is needed and how much it will cost. All the details should be objectively presented and permanently documented.

In Bern, there were large failures in protection of the historical heritage before the WWII, but the mentioned measures, introduced seven to eight decades ago, considerably changed this situation. This change is manifested primarily in gradual but continuous monitoring of the monuments, timely repairs so that the failures are unlikely to happen again.

2.6. Making an inventory of the cultural monuments as a preventive protection measure

For very long time, only some public and sacral buildings were listed and marked as cultural monuments in Bern. The examples are the Town Palace, which is one of the oldest palaces of the historical part of Bern, which was by the owner and in certain political circles marked as an important structure in the memory of the city of Bern, as well as the Cathedral (*Münster*) whose construction began in 1421 as the tallest cathedral in Switzerland. The number of cultural monuments carefully and gradually expanded.

The Historical Protection Commission with great care and attention prepares and composes the reports, so that they are readily understandable to the people who are not experts, so that they will be acquainted with the works to be executed on the cultural monuments. Yet, these lists sometimes fail to meet all the requirements and therefore, there has been an intensive effort in the past twenty years to evaluate the building heritage, so that it could be entered in an inventory. Simultaneously, this inventory is digitalized in the central data bank, according to all federal, cantonal and city laws. Since the beginning of work, the Commission endeavoured to have its activities accepted and understood by the citizens of Bern, since the citizens pay for the operation of the Commission, through various forms of taxes. Several years ago, the citizens' initiatives establishments succeeded in gaining right to decide and affect decision making on the occasion of including listed buildings in the Inventory list.

The forms of citizens' initiatives got the green light in the ministries of the Bern Canton and the citizens decision making rights were brought to a higher level. In practice, the system work comprises the Committee making a list of the buildings, classifying them, determining the protection category; but if the Committee, based on the professional expertise, decides to demolish the cultural monument, or to extensively renovate it, regardless of the high cost for the tax payers, then the citizens come forth with their initiative. The citizens' initiative is mechanism that is used to the maximum in Switzerland and thus in Bern. As soon as the citizens conclude that they should react they gather signatures and put the issue on a referendum whether to accept the Committee decision or not.

The referendum results are binding for all the participants of the process. With this decision-making mechanism, the processes are beneficial for both sites. In this way the work of the Committee is assessed, i.e.

ne spomenike kulture samo je jedan od mnogih faktora. Može se dogoditi da interesi političkih vlasti i glasača mogu vredeti manje od ekonomskih interesa i da se zaštita spomenika kulture može odbiti.

Svaki potencijalni sukob u planskom procesu treba da se izbegne ili da se odloži za neko vreme, da bi se o tome raspravljalo na nekim kasnijim sastancima. Iz perspektive zaštite, veoma je važno da se sačini spisak zgrada koje nisu od izuzetne istorijske vrednosti. Ipak, prilikom budućeg korišćenja tih objekata vodi se briga ne samo o finansijskoj isplativosti, nego i o kvalitetu zaštite njihovih istorijskih osobina. Primer za to jeste provizija koja se dobija po završetku zadatka. Kad je reč o najvrednijim spomenicima i njihovim vrednostima, konsenzus o dugoročnoj zaštiti treba da bude obezbeđen. Efektivni načini preventivne konzervacije su u Bernu, nažalost, ograničeni.

Kao deo Naredbe o izgradnji, vlasnici finansija će uživati korist kada se određena zgrada ne koristi, pa se restaurira i dobije novu namenu. U Uredbi Zavoda za izgradnju grada za korišćenje javnog zemljišta na kojem se nalaze spomenici kulture stoji da polovinu troškova plaćaju poreski obveznici, a ta sredstva se koriste za restauraciju zgrada, te se tako izbegava dodatno finansiranje iz budžeta građana.

Preventivna konzervacija predstavlja reakciju koja je proizašla iz specifičnih okolnosti. Istorijska konzervacija ima mnogo nivoa i mnogo alata za preventivne mere zaštite. Tokom poslednjih dvadeset godina, pokušavajući da se zaštiti, grad Bern je usvojio mnoge od tih metoda i sproveo ih u praksu. Deo procesa je završen, ali konačan cilj još nije dostignut.

3.1 Idilična zaštita okoline u gradu Bernu

Grad Bern je 1979. godine formirao posebnu gradsku Agenciju za istorijsku zaštitu. To je bio odgovor na zabrinutost države i Bernskog kantonalnog konzervatora, pogotovo zbog ubeđenja gradske vlasti i građana u to da je stari grad Bern, kao celina, baština međunarodne vrednosti i da je intenzivna zaštita preko potrebna. Međutim, već od početka, rad agencije nije bio ograničen samo na Stari grad, uključene su takođe i važne istorijske zgrade u predgrađu, a kasnije i vredne građevine modernog doba. Podela odgovornosti i rada određena je geografskom podelom, a za sve poslove konačno je nadležna Gradska stručna komisija, bez obzira na kategorizaciju objekata.

Bliska saradnja s kantonalnom službom za preventivnu zaštitu umnogome je doprinela boljoj brizi za nasleđe. S ciljem da bude što manje nepravilnosti, doneta je odluka da Gradska konzervatorska komisija bude jedina odgovorna za sve finansijske kredite budućih projekata u oblasti zaštite. Na taj način, jednom peticijom, odnosno prigovorom, moguće je prijaviti nepravilnosti u radu i kontroli transparentnosti rada ove komisije. Jasno definisanom metodom obračuna servisnih usluga od strane Udruženja švajcarskih zaštitara, omogućeno je da se izračuna čist, transparentno vidljiv doprinos koji mora da se obezbedi.[20] Pod vođstvom različitih kreditnih agencija, doprinosi su počeli da se uplaćuju. Doprinosi su jedan od elemenata na kojima se bazira rad Agencije za zaštitu grada Berna, iako je ona,

monuments of outstanding value is only one of many factors. It may occur that the interest of political authorities and voters may be outweighed by the economic interest, and that protection of cultural monuments is rejected.

Every potential conflict in the planning process should be avoided or postponed for a certain time, so that it could be discussed in some later meetings. From the perspective of protection, it is very important to make a list of buildings which are not of special historical value. Yet, in the future use of these buildings, care is taken not only about the financial profitability but also about the quality of protection of their historical characteristics. An example for that is the fee obtained upon completion of the task. When the most valuable monuments are concerned, the consensus on the long-term protection should be provided. The efficient methods of preventive conservation in Bern are, unfortunately, limited.

As a part of the Building order, the financiers can make profit in cases when a certain building is disused and is then restored and put to a new use. The decree of the City building Agency prescribes that half of the costs of the land occupied by the cultural monuments is borne by the tax payers, and those resources are used for buildings restoration, and thus the additional financing from the citizens' budget is avoided. Preventive conservation represents a reaction resulting from the specific circumstances. Historical conservation has multiple levels and tools for preventive protection measures. During the last twenty years, in an attempt to protect itself, the city of Bern adopted many methods and put them in practice. A part of the process is completed, but the final goal has not yet been achieved.

3.1 Idyllically protection of the environment in the city of Bern

The city of Bern established a special city protection agency in 1979. It was a response caused by the concern of the state and the cantonal conservationist of Bern, but also by the deep conviction of the city authorities and citizens that the old town of Bern as a whole was a heritage of an international importance and that the intensive protection was required. However, from the outset, the work of the agency was unlimited to the old town, included were many important historical buildings in the suburbs, and in the later period some valuable modern age building. The division of competences and work is determined by the geographic division, and the Professional city Committee is ultimately competent for all the works irrespective of the building categorization.

Close cooperation with the cantonal service for preventive protection largely contributed to a better level of care about of the built heritage. A decision was made to make the city's Historical Protection Commission be solely responsible for all the financial credits of the future protection projects. Thus, by only one petition, i.e. objection, it is possible to report the irregularities in the work and control of the work of this Commission. Due to the clearly defined method of billing of services by the Swiss Agency for the Protection of Cultural Property it is possible to calculate a clear and transparent contribution which should be provided. [20] Under guidance of

uglavnom, nadzorno i savetodavno telo.

Agencija, takođe, nadgleda projekte za glavne poslove, kao što je formiranje inventara spomenika kulture. Predsedavajući u ovoj agenciji je konzervator iz Komisije za istorijsku zaštitu. Ova komisija svake dve godine dodeljuje nagradu *dr Jost-Hartmann*, za „najbolje revitalizovanu i zaštićenu kuću u Starom Bernu”. [22]

Nagrađuju se osobe koje su, preduzimajući restauraciju, sledile principe zaštite i očuvale integritet vredne građevinske strukture. Nagrade se dodeljuju arhitektima i građevinskim koncernima, inženjerima i restauratorima, zanatlijama, ali i zaslužnim političarima. Članovi Agencije godinama se ne menjaju, da bi se očuvali ustanovljeni kriterijumi. Imajući u vidu broj spomenika kulture koji su pod njihovom brigom, njihovi uslovi za delovanje su skromni. Poslednje četiri godine izvori finansija su se smanjili, uglavnom za prateću administraciju. [22]

3.2 Četiri izveštaja o preventivnoj zaštiti grada Berna

U petom setu četvorogodišnjih izveštaja o preventivnoj zaštiti, koji pokriva period od 1997. do 2000. godine, uočavaju se mogućnosti da se rad Agencije predstavi široj javnosti, da bi se opisali glavni doprinosi, uslovi, kao i razne poteškoće s kojima se članovi u radu susreću. [24] Dugoročni planovi i trendovi takođe su uključeni u izveštaje. Ovaj četvorogodišnji izveštaj objavljuje se u „Bernskom magazinu istorije i lokalne istorije”. Prikazani primeri, odnosno zaštićene zgrade predstavnici su svih funkcionalnih oblika i služe kao uzori za mnogobrojne potonje restauracije, u kojima se javljaju slični problemi.

Pomoću tih primera uspešnih restauracija i konzervacija, uspostavljaju se opšteprihvaćene smernice za istorijsku konzervaciju. Pojedinačni autori, vlasnici zgrada i arhitekta, obrazlažu svoje lične primere angažovanja. Većina tekstova odnosi se na određivanje vrednosti spomenika kulture. Uočava se i učešće onih koji nisu neposredno vezani ličnim interesom za neki spomenik kulture, ali osećaju moralnu ili profesionalnu obavezu da skrenu pažnju na vrednosti koje zapažaju. [5] U arhivima se čuva i obiman materijal, ne samo o spomenicima kulture, već i dokumentacija o restauratorskim radovima koji su na njima do sada obavljani. Ova dokumentacija je svima dostupna, a njen dodatak predstavlja i zvanični detaljni interni izveštaj. U tom izveštaju navedeni su nerešeni problemi u pogledu istorijata zgrada, njenog nastanka i promena tokom životnog veka, kao i posebni problemi koji su uočeni tokom restauracije. U godišnjim izveštajima grada Berna nalaze se, takođe, lista objekata na kojima su izvršeni restauratorski radovi, ali i lista projekata koji su odbačeni. Najdramatičniji događaj iz perioda koji pokrivaju ovi izveštaji jeste nesreća u ulici *Randbjunkergasse*, početkom 1997. godine, kada se nekoliko kuća srušilo. Velike štete utvrđene su na pet

various credit agencies, the contributions began to pay off. The contributions are one of the elements on which the operation of the Agency for Protection of the City of Bern is based, even though it is mostly concerned with supervision and consulting.

The Agency also supervises the projects of the major tasks, such as establishing the cultural monuments inventory. The chairman of the Agency is a conservationist belonging to the Historical Protection Commission, and the other members are respectable professionals in fields relevant for the cultural property protection. The Commission biannually awards the *dr Jost-Hartmann* prize, for the best revitalized and protected house in Old Bern”. [22]

The prizes are awarded to the individuals who, undertaking restoration, followed the protection principles and preserved the integrity of a valuable building structure. The buildings are awarded to architects and contracting concerns, engineers and restorers, craftsmen and politicians. The members of the Commission remain the same for years, in order to retain the established criteria. In comparison to the number of cultural monuments in their care, their working conditions are modest. In the last four years, the financial resources were reduced, mostly for the accompanying administration. [22]

3.2 Four reports on the preventive protection of the city of Bern

In the fifth set of four-year reports on preventive protection, covering the period 1997-2000, there is potential to present the work of the Agency to the wide public, in order to describe the main contributions, conditions and various difficulties the members encounter in their work. [24] The long-term plans and trends are also included in the reports. This four year report is published in the “Bern journal of history and local history”. The presented examples, i.e. the protected buildings are representatives of all the functional forms and they serve as models for numerous subsequent restorations that feature similar problems.

With the aid of these examples of successful restorations and conservations, generally accepted guidelines for historical conservation are established. Individual authors, such as the building owners and architects explain their personal actions. The most of the texts refer to the determination of the values of the cultural monuments. Also notable is the participation of those who are not personally involved with a cultural monument, but who feel the moral or professional obligation to draw the public attention to the values they perceive. [5] Extensive material is being kept in the archives, concerning not only cultural monuments, but documents on the restoration works performed on them up to now. These documents are widely available. An official detailed internal report is an addition to these documents. The report lists the unsolved problems concerning the historical background of the buildings, their construction and changes they underwent during their life time, as well as special problems detected during the restoration. The yearly reports of the city of Bern also comprise the list of buildings which underwent the restoration works, but also the list of rejected designs. The most dramatic event from the period

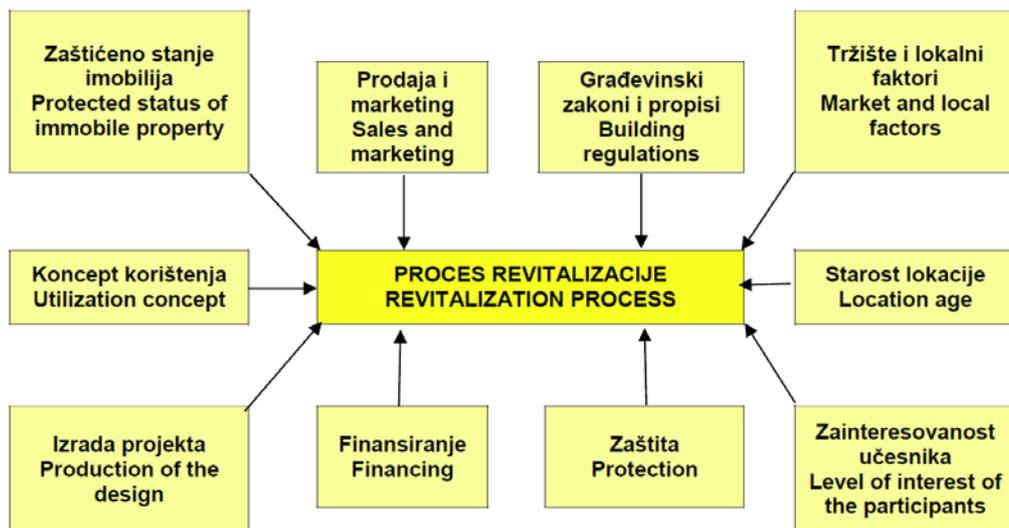
koje je vlasništvo pojedinca, već je bitno za opšte i javno dobro celokupnog društva u celini. Ono se identifikuje, proučava i prenosi na buduće generacije tako što se beleži, evidentira, štiti i ostavlja za dobrobit celokupnom švajcarskom društvu i celom svetu.

Graditeljsko nasleđe odražava regionalnost, različite tipove građevina i tradicije uopšte. Ono može da obuhvata: od farme do velikog grada, nadogradnju, komercijalne industrijske komplekse, radnička naselja, zamkove, kao i arheološka nalazišta. Spomenici kulture nisu samo arhitektonski objekti iznad prosečnog kvaliteta, već to mogu biti i jednostavni, ali za društvo važni istorijski subjekti koji doprinose uravnoteženju današnje slike gradova. Graditeljsko nasleđe podrazumeva sve što je u prošlosti sagrađeno, a ima spomeničke vrednosti. Sve što je proglašeno graditeljskim nasleđem mora biti zaštićeno, a ako je zaštićeno, onda **mora biti sačuvano.**[21]

4 FAKTORI KOJI UTIČU NA PROCES REVITALIZACIJE

Revitalizacija nekorišćenih istorijskih spomenika jeste proces u koji je uključeno nekoliko odlučujućih faktora. Ti faktori su konstante i utiču na pojedine faze revitalizacije. Sama revitalizacija nije jednoznačno određena, pošto se spomenici kulture razlikuju po svojim karakteristikama i lokacijama. Ipak, mogu se postaviti određene smernice – kao svojevrsni akcioni vodič koji se, potom, prilagođava pojedinačnim slučajevima.

Faktori koji utiču na proces revitalizacije nekorišćenih, odnosno napuštenih, spomenika kulture i njihove međusobne veze prikazani su na Shemi 1.



Shema 1. Faktori koji utiču na proces revitalizacije
Scheme 1. Factors influencing the revitalization process

Faktori, osim što su stalno prisutni, pomažu i u pojedinačnim fazama rada. Zbog toga se veze između faktora na grafikonu mogu koristiti kao preporuka prilikom praktičnog rada. Neke faktore, do određenog stepena, mogu kontrolisati razni učesnici. Pojedini, međutim, moraju se usvojiti takvi kakvi jesu. To su,

individuals, but is important public and general resource of the entire society. It is identified, studied and passed on to the future generations, by being recorded, listed and left to the benefit of the entire Swiss society and the world.

The built heritage reflects regional traits, various types of buildings and tradition in general. It can include: farms and big cities, extended buildings, commercial industrial complexes, working people districts, castles and archeological sites. The cultural monuments are not only exceptional quality architecture, but those can be simple, but socially important historical subjects which characterize and contribute to the balancing of the contemporary image of the cities. The built heritage comprises everything built in the past, possessing the monumental value. Every item declared building heritage must be listed, and if listed, it **must be preserved.** [21]

4 FACTORS INFLUENCING THE PROCESS OF REVITALIZATION

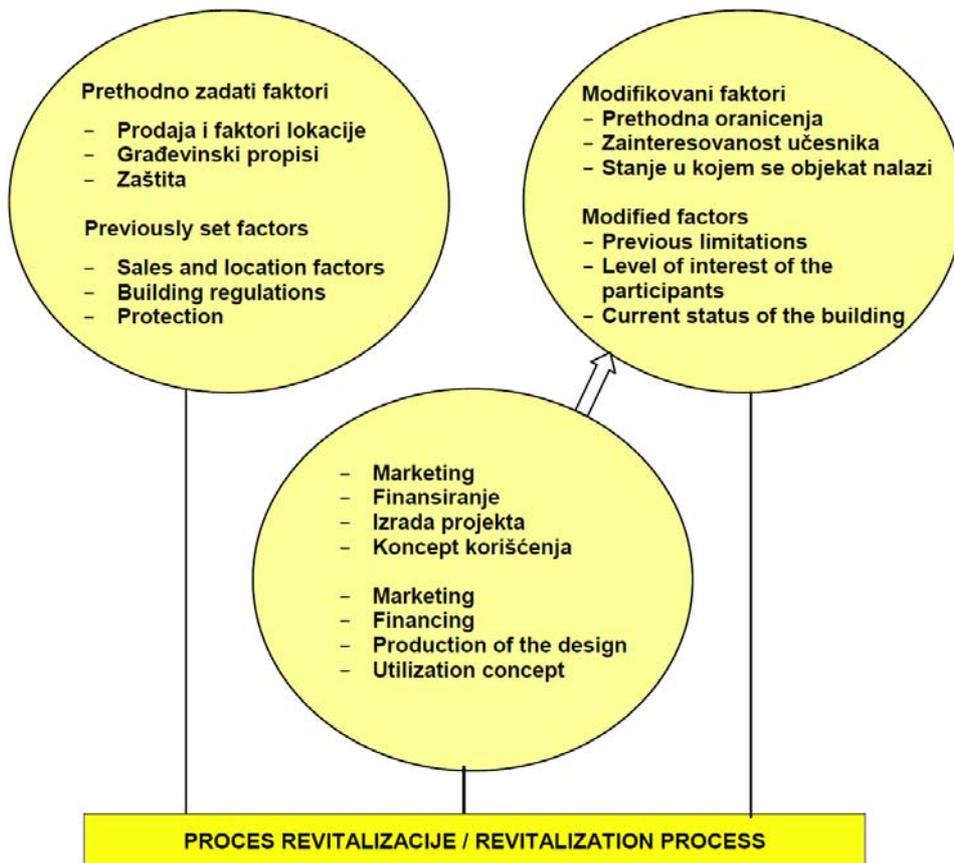
Revitalization of disused historical monuments is a process which includes several decisive factors. Those factors are constantly present and they affect individual phases of revitalization. The process itself is not universal, since the cultural monuments differ in terms of their characteristics and locations. Yet, certain guidelines can be provided as certain action guide, which is, subsequently adapted to the individual cases.

The factors affecting the revitalization process of unused, i.e. abandoned cultural monuments and their mutual relations are displayed in Scheme 1.

The factors, apart from being ever present, help in the individual work phases. For that reason, the relations between the factors in the chart can be used as a recommendation in the practical work. Some factors can be controlled to a certain degree by the various participants. Some of them, however, should be adopted

uglavnom, faktori tržišta i lokacije. Na druge faktore, kao, na primer, postojeće stanje nekretnina, može se uticati. Pri revitalizaciji, mora se voditi računa o vodećim tačkama u procesu i ograničenjima koje treba poštovati. Shema 2. prikazuje preporuke, u obliku akcionog plana, koje značajno unapređuju kvalitet revitalizacije.

as they are. Those are, prevalently, the market and location factors. The other factors, such as, for instance, the current status of the real estates, can be affected. While performing revitalization, one should take care about the major points of the process and of the limitations to be observed. The following scheme displays the recommendations, in the form of an action plan, which considerably improves the revitalization process quality.



Shema 2. Preporuke za uspešan proces revitalizacije [21]
 Scheme 2. Recommendations of successful revitalization process [21]

Akcioni plan prvenstveno uključuje aktivne dizajnerske faktore. Za održivost projekta bitna je ujednačenost svih faktora koji najviše utiču na revitalizaciju. Zbog toga je veoma važno prepoznavanje i pronalaženje novih ideja prilikom dodeljivanja nove namene objekata.

The action plan primarily involves the active designing factors. The project sustainability requires balance and uniformity of all the factors which mostly affect the process. For that reason, recognition and finding new ideas when converting the buildings is very important.

4.1 Interesi u procesu revitalizacije i akcionari

4.1 Diverse interests in the revitalization process and shareholders

Analiza moguće namene praznih, napuštenih istorijskih građevina jedna je od najvažnijih faza revitalizacije.

An analysis of the potential use of the empty, abandoned historical buildings is one of the most important phases of revitalization.

Početak izrade projektnog plana zasniva se na pretpostavkama da se u procesu revitalizacije originalna, izvorna namena i struktura zgrade mogu donekle izmeniti, u zavisnosti od njene procenjene istorijske, estetske, arhitektonske i konstruktivne, te društvene vrednosti. Kada se utvrdi taj obim mogućih izmena, nadalje o tim ograničenjima nadležnost preuzima menadžer objekta. U zavisnosti od veličine građevine i razloga za njenu

The onset of production of the project plan is based on the assumptions that in the revitalization process, the original function and structure of the building can be changed to a certain degree, depending on its assessed historical, esthetic, architectonic and structural, as well as social value. When the scope of potential changes

revitalizaciju, cilj revitalizacije može se ostvariti uz pomoć specijalizovanog graditelja ili arhitekta, ukoliko je reč o manjim objektima. Preporuke i sugestije za formiranje veza u različitim razvojnim fazama revitalizacije prikazane su na Shemi 3.

Revitalizacija se u ovoj shemi definiše kao deo koncepta, ali deo koji se odnosi na neiskorišćenost same građevine može biti dodatno od pomoći.

Uglavnom, cilj je da se izradi analiza namene i da se koordiniraju akcionari, odnosno deoničari koji učestvuju u revitalizaciji. Namera zainteresovanih strana jeste da se projekat izloži njihovim pretpostavljenima, menadžerima projekta ili planerima. Primena ove analize omogućava pokretanje projekta.

Interes vlasnika napuštenog, praznog objekta, jeste da ga što bolje proda, odnosno, ukoliko ga zadrži u svom vlasništvu, da ga iznajmi na što duži period.

Interes izvođača radova i investitora jeste sticanje profita, što projektom revitalizacije, odnosno uvođenjem nove, odgovarajuće namene, može da se ostvari. U izboru odgovarajuće, najprimerenije namene mogu da učestvuju sve zainteresovane strane. Prilikom izbora, mora se voditi računa o tome da se ostvari harmonija između postojećih karakteristika objekta koje čine njegovu vrednost, i zahteva koje postavlja buduća namena objekta.

Dobro poznavanje prednosti revitalizacije leži u prethodnom poznavanju dve vrste parametara - onih kojima se određuju dodatni poslovi i onih u pogledu ograničenja usled zaštićenih vrednosti građevine, koji mogu biti preovlađujući faktor pri odluci da se investira u projekat revitalizacije.

Za korisnike objekta važno je da znaju koja su ograničenja objekta prilikom odabira buduće namene. Ograničenja koja utiču na novu namenu korisnicima saopštava menadžer objekta. Prethodno, on može da koristi analize o nameni i strategijskom planiranju, te da to uključi u planiranje, koje obuhvata i strategijsko upravljanje objektom. Za arhitektu-konzervatora, koji je predstavnik javnog interesa, glavna prednost koju ima od analiza jeste to što, pomoću njih, minimalizuje gubitke, zbog različitih svojstava istorijske zgrade, tokom prilagođavanja objekta budućoj nameni.

has been determined, in the further process the manager of the building is competent for these limitations. Depending on the size of the building, and reasons for its revitalization, the goal of the revitalization can be achieved with the aid of the specialized builder or architect, if small size buildings are concerned. The recommendations and suggestions for formation of relations in various phases of revitalization development are displayed in scheme 3.

Revitalization is in this scheme defined as a part of the concept, but the part referring to the unused capacity of the building itself can be of additional help.

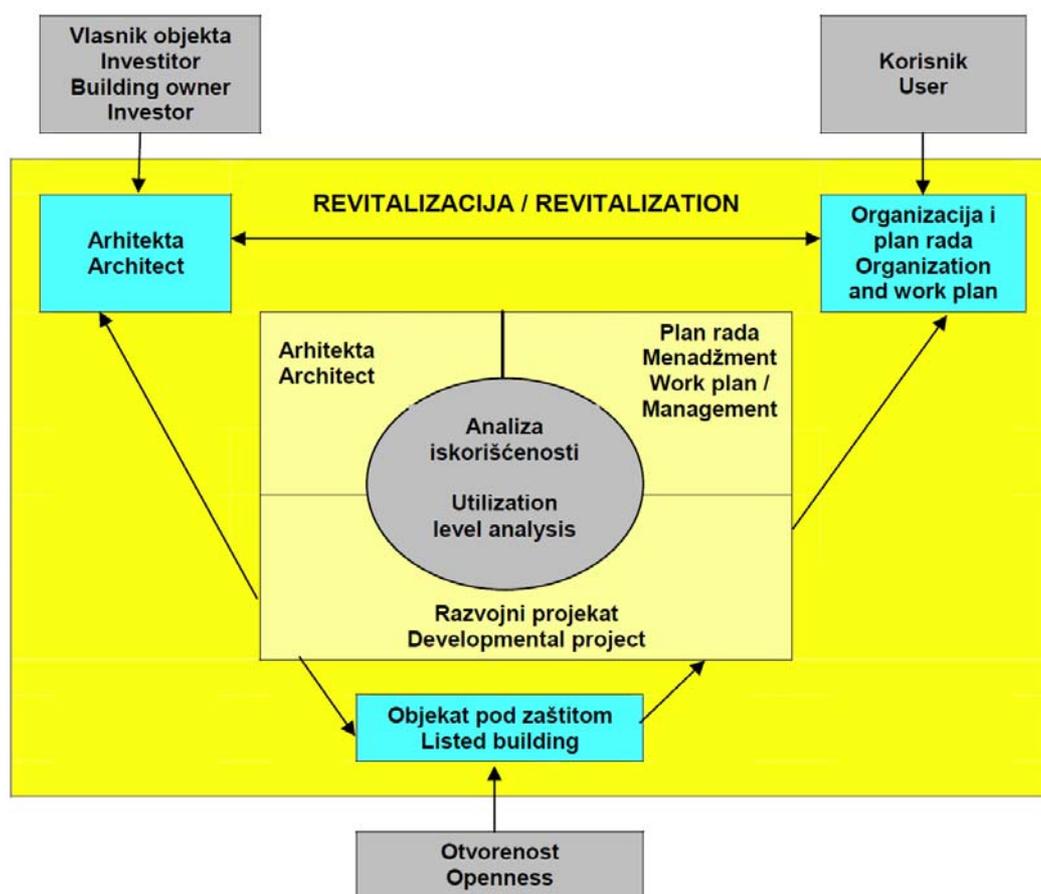
The goal is, prevalently, to make an analysis of the use, and coordinate the shareholders taking part in the revitalization. The intention of the stakeholders is to display the project to their superiors, project managers or planners. The application of this analysis makes it possible to start the project.

The interest of the owner of an abandoned, i.e. empty building is to sell it with the best profit, or, if the ownership over the building is retained, to attempt to rent it for as long period as possible.

The interest of the contractor and investor is to make profit, which can be achieved through the revitalization project, i.e. through introduction of a new, appropriate use. All the stakeholders can participate in the choice of the most appropriate use. When making a choice one should take care to create harmony between the current characteristics of the building, which constitute its value, on one hand, and the requirements posed by the future use of the building, on the other hand.

Good knowledge of the advantages of revitalization comprises previous knowledge of two types of parameters, one type determining the additional works, and the other related to the limitations due to the protected values of the building, which can be a prevailing factor when making a decision to invest in the revitalization project.

When choosing the future use, it is important for the users of the building to know what the limitations of the building are. The users are informed about the limitations affecting the new use from the building manager. He can previously utilize the analyses of the use and strategic planning, and include it in the planning process, which comprises the strategic management of the building. For and architect-conservationist, who represents the public interest, the main advantage of the analyses is that, using them, he can minimize the losses related to the various properties of the historical buildings, in the process of adapting the building to the future use.



Shema 4. Učesnici u procesu revitalizacije i njihove veze [21]
 Scheme 4. Participants in the revitalization process and their relations [21]

4.2 Promene na tržištu – ponovno otkrivanje istorijskih vrednosti objekata – odnos korisnika i investitora

Na tržištu nekretnina, rokovi za izradu projekata veoma su kratki i nikada nema dovoljno vremena da se o njima ozbiljno promisli. Umesto „proizvoda – (rezultata) razmišljanja” [4] na početku revitalizacije na gradilištima nisu postojali nikakvi istaknuti podaci kojima se, atraktivnim reklamama, privlače potencijalni korisnici, odnosno investitori.

Tradicionalno *teški* faktori, koji se, po pravilu, povezuju s lokacijom, odnosno s dobiti uslovljenom infrastrukturom, bili bi pod znakom pitanja, a formulisanje novih *mekih* kriterijuma lokacije, na prvi pogled, manje atraktivnih, bili bi u drugim slučajevima zanemareni.”[4]

Osnovni principi kojima se rukovode moderni građevinski projekt jesu veća orijentacija prema korisnicima (tržišna orijentacija), te veća osetljivost u pogledu situacije (orijentacija prema okruženju). [4] Kada su zaštićene zgrade u pitanju, one imaju koristi od ovih faktora, jer se, u njihovom slučaju, primenjuju *mekani* faktori.

U „Spomeničkoj studiji Berna” izneti su podaci ankete u kojoj su korisnici i investitori ispitani o njihovom

4.2 Changes in the market – rediscovery of the historical values of the buildings – relations of the users and investors

In the real estate market, the deadlines for production of projects are very short, and there is never sufficient time to dwell upon them. Instead of “the product – (result) of deep thinking” at the beginning of revitalization, there were no displayed data on the construction sites, which could, as an attractive advertisement, attract potential users, i.e. investors.

„The traditional *heavy* factors, which are, as a rule, related to the location, that is, to the profits related to the infrastructure, would be questioned, and formulation of new *soft* criteria of the location, which are, at first sight, less attractive, would be neglected.”[4]

The basic principles of contemporary construction projects are increased orientation towards the users (market oriented projects), and an increased sensitivity in respect to the situation (environment oriented projects). [4] When the listed buildings are concerned, they can benefit from these principles, since in their case, the *soft* factors are implemented.

In “The Monumental study of Bern” the data from the survey in which the users and the investors were questioned about their experience in revitalization and the

Materijali i konstrukcija ukazuju na to kako je zgrada građena i na vreme u kojem je građena. Ovo se odnosi kako na celokupni objekat, tako i na njegove pojedinačne komponente. Funkcija određuje praktičnu vrednost objekta, odnosno njegovu moguću savremenu upotrebljivost. Ovakav pristup dovodi nas do teze da procesi na građevini, ukoliko se promišljeno sprovode, odnosno ukoliko se pravilno sagleda i vrednuje prilagodljivost oblika, konstrukcije, materijala i prostorne organizacije, do mere koja ne ugrožava spomeničku vrednost, mogu da generišu novu namenu i nastavak života devastirane građevine.

4.4 Korisne namene zaštićenih objekata

Odnos između investitora i konzervatora uvek je bio zategnut, kada se imaju u vidu i vrednosti nekretnina i spomenička vrednost kulturnog nasleđa. S jedne strane, subjektivni stavovi investitora koji žele da imaju visok povračaj profita, a s druge strane održavanje istorijske vrednosti spomenika kulture – stalno su u sukobu. Najčešći sukobi oko očuvanja spomenika kulture jesu, na primer:[10]

- Kratkoročni ekonomski modeli, koji nisu zasnovani na principima spomeničke korisnosti, nisu pogodni za očuvanje istorijskih zgrada (brz profit na uložene investicije korišćenjem vlasništva);

- Zakoni i standardi zasnovani su na principima izgradnje i eksploatacije novih, savremenih zgrada. Kada su spomenici kulture u pitanju, mnogi od zakona i standarda ne mogu da se slede da se ne bi prouzrokovala šteta u procesu rehabilitacije spomenika;

- Preobimno korišćenje zgrade, kada se, na primer, preterano preoblikuje potkrovlje, često značajno narušava originalnost objekta.

Jedino interesi korisnika istorijskih objekata mogu da dovedu do promene u tržištu nekretnina. Švajcarske publikacije kao što su „Spomenik kao vlasništvo” [9] ili „Spomenik kao investicija”[1] ukazuju na sporo usklađivanje i povezivanje upotrebe spomenika kulture kao nekretnine i njegovog istorijskog očuvanja. Da bi se naglasio značaj istorijskog spomenika, koji ne treba posmatrati kao praznu školjku, sada je uveden izraz „upravljanje istorijskim spomenicima”. Pored toga, treba imati stalno na umu da vrednost spomenika kulture nije samo u njegovom spoljašnjem izgledu, što ponekad zanemaruju i stručnjaci koji se neposredno bave zaštitom kulturne baštine.[10]

4.5 Mogućnosti i rizici revitalizacije istorijskih zgrada

Na osnovu iznetog, može se zaključiti da korisnici i investitori, ipak, pozitivno prihvataju korišćenje istorijskih, zaštićenih, građevina. U daljem razvoju projekta revitalizacije, međutim, postoje određeni rizici. Na primer:[3]

- Lokacije istorijskih objekata dobro su dokumentovane i određene, ali skupa infrastruktura, koja

Every building has a certain structure, manifested through: function, form, material and structure.

Materials and structure reflect the building process of the time in which it was built. This refers both to the entire buildings and its individual components. Function determines the practical value of the building, that is, its potential contemporary serviceability. Such approach promotes a thesis that the processes on the building can generate a new use and continuation of life of a dilapidated building, if it is conducted in a well-conceived manner, i.e. if the adaptability of forms, structure, materials and spatial organization is appropriately considered and valued, so as not to endanger the monumental value.

4.4 Practical uses of listed buildings

There has always been a tense relationship between the investors and conservationists when the ratio of the property value and monumental value of the cultural heritage is considered. Subjective attitudes of the investors who want to have a high profit return on one hand, and conservation of the historical value of the cultural monument, on the other, are in permanent conflict. The most common conflicts regarding the preservation of cultural monuments are, for instance: [10]

- Short term economical models which are not based on the principles of monumental utility, are not suitable for preservation of historical buildings (fast profit from the invested finances, through utilization of ownership);

- The laws and standards are based on the principles of construction and usage of new, contemporary buildings. When this concerns the cultural monuments, many of the laws and standards cannot be observed since they will cause damage in the process of monument rehabilitation;

- Excessive usage of the building, for example, when the attic is excessively altered often causes considerable loss of building originality.

Only the interests of the users of historical buildings can bring about change in the real estate market. The Swiss publications, such as a “Monuments as a property” [9] or „Monument as an investment” [1] indicate a slow harmonization and linking of the cultural monument use, as an immobile property, with its historical preservation. In order to emphasize the importance of the historical movement, which should not be viewed as an empty shell, the term “historical monument management” has been introduced. After that, one should constantly bear in mind that the value of the cultural monuments is not in their external appearance, which is at times overseen even by the professional engaging directly in the cultural heritage protection. [10]

4.5 Potential and risks of historical building revitalization

On the basis of the discussion, one can conclude that, usage of historical, listed buildings has been positively accepted by the users and investors. In the further development of revitalization project, however, there are certain risks, such as, for instance: [3]

- Locations of historical buildings are well documented and determined, but the expensive

- rizik vremena, odnosno prognoziranje trajanja revitalizacije;
- rizik razvoja.

5 ZAKLJUČNA RAZMATRANJA I PREPORUKE

Najvažniji rezultati uspešne revitalizacije, prema ovom švajcarskom istraživanju i iskustvu, jesu sledeći:

– Proces revitalizacije treba proučavati u kontekstu ciklusa životnog veka zaštićenog spomenika kulture. Postojeći koncept revitalizacije proširen je na intenzivno uvođenje novih namena u napuštene istorijske objekte, sa idejom da se ponovno koriste, a ne da se uklanjaju.

– Faktori uticaja određeni su kao privremeni, ali su uključeni u analize koje su dalje razvijane i na osnovu kojih su preporučeni pravci implementacije revitalizacije na zaštićenim spomenicima kulture.

– Rezultati revitalizacije zavise od uvođenja nekoliko analiza, koje nisu uobičajene prilikom ustaljenog procesa obnove istorijskih spomenika kulture. Jedna od takvih analiza jeste analiza korišćenja koja se uključuje u koncept analize celokupne revitalizacije zaštićenih spomenika kulture, a zavisi od istorijske metode vrednovanja spomenika u samom procesu.

Ova analiza osmišljena je da se primeni prvenstveno za nekretnine – spomenike kulture, koji zbog istorijske upotrebe i bez specijalnih prilagođavanja, veoma teško mogu da budu predmet interesovanja potencijalnih korisnika, bilo da je reč o iznajmljivanju ili kupovini, te usled toga sve više propadaju.

– Za istorijske objekte koji zadržavaju svoju *namen*, na sadašnjem tržištu nekretnina veoma je teško da nađu potencijalnog novog korisnika. *Analiza namene*, naročito za stambene objekte, dosta je skupa. Kao primer može poslužiti bivša Ženska bolnica u Bernu, za čiju zgradu je analizom namene otkriven veliki upotrebnost potencijal, pa je prostorna organizacija prilagođena potrebama Univerziteta u Bernu, koji je tako dobio značajno proširenje u neposrednoj blizini već postojećih kapaciteta.

Analiza namene za ovaj primer veoma uspešno je sprovedena u dve faze, sukcesivno izvedene:

Faza 1 obuhvatila je trenutnu mogućnost vlasnika da identifikacijom potencijalne namene objekta cilja na specifičnu grupu mogućih korisnika. Zbog toga je primena marketinga u ovoj fazi izuzetno značajna. Identifikacija buduće namene napuštenih istorijskih građevina jeste sistematski proces, zasnovan na metodi otvorenog pristupa, odnosno kreativnoj metodi *brainstorm*, tokom koje svi zainteresovani sarađuju i razmenjuju ideje. U svojevrstnoj kreativnoj radionici poštuju se neki parametri koji imaju u vidu istorijsku vrednost građevine i delovanje različitih uticaja na objekat. Delovanje ovih parametara mora da bude fleksibilno, to jest da u okviru određenih granica mogu da se prilagođavaju. Moderator mora da omogući učesnicima da stvore realnu sliku o istorijskim karakteristikama objekta (npr. arhitektonskim, konstrukcijskim, stilskim). Ovi parametri služe da se zgrada dobro proceni u svojoj ukupnosti i obradi u skladu sa analizama predviđenim u procesu revitalizacije.

- Financial risk of investment in revitalization;
- Project cost and works execution risk, due to the unpredictable activities;
- Risk of time, that is predictions of revitalization duration;
- Development risk

5 CONCLUSION REMARKS AND RECOMMENDATIONS

The most important results of a successful revitalization according to this Swiss research and experience are as follows:

– revitalization process should be studied in the context of the life cycle of a listed cultural monument. The existing concept of revitalization is extended to the intensive introduction of new uses into the abandoned historical buildings, following the idea to use them again, and not to remove them.

– the influence factors are defined as temporary, but involved in the analyses which are further developed and which served as a basis for making the recommendations about directions of implementation of revitalization of the listed cultural monuments.

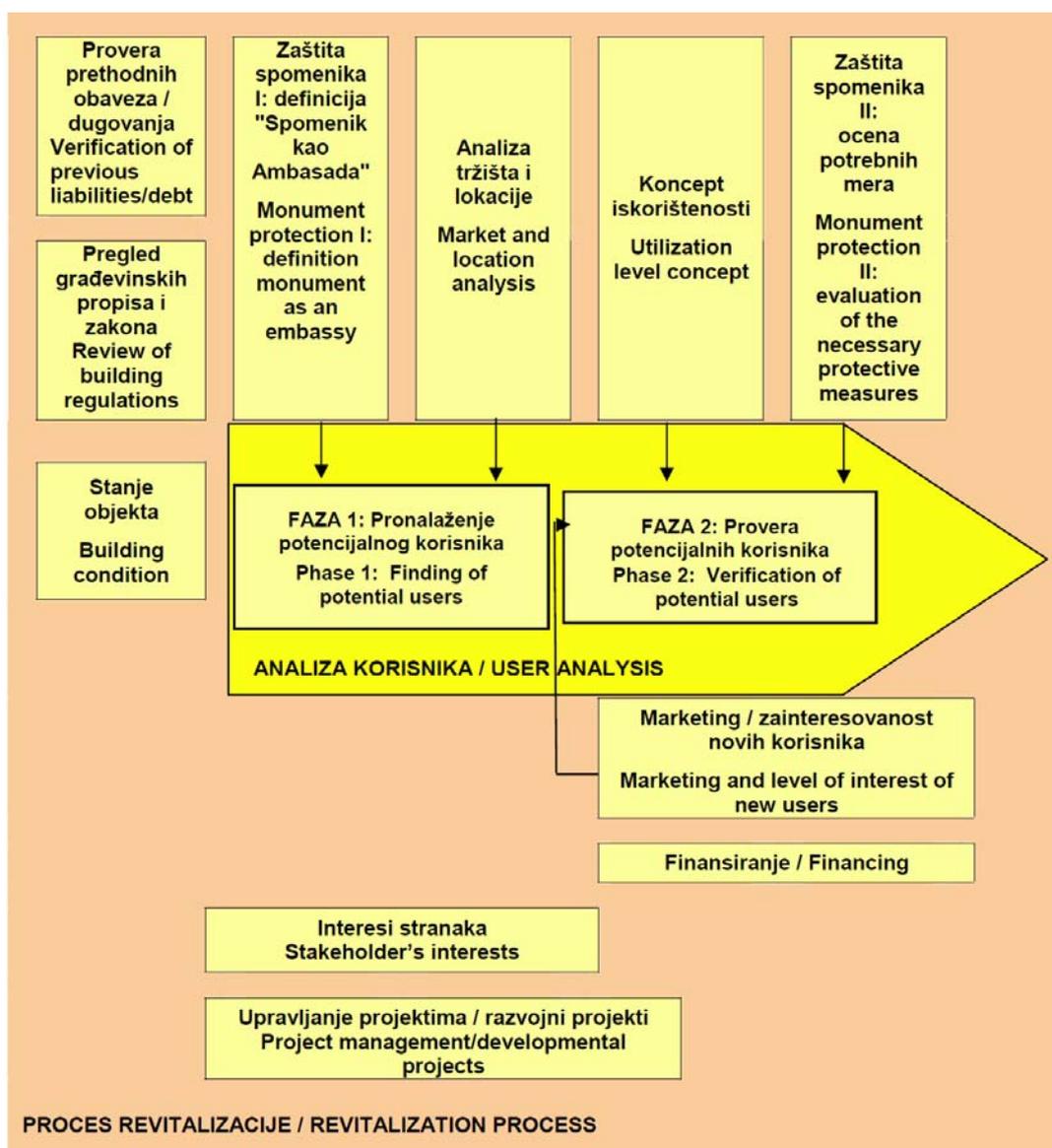
– the revitalization results depend on the introduction of several analyses which are not common in the regular process of revitalization of historical cultural monuments. One of such analyses is the analysis of utilization, which becomes a part of the concept of analysis of entire revitalization of the protected cultural monuments, and which depends on the historical method of assessment of monuments in the process itself.

This analysis is conceived so as to be implemented for the immovable property, historical monuments, which through historical use and without special adaptations can hardly be interesting for potential users, either in terms of renting or purchasing, and which consequently continue to deteriorate further.

– It is very difficult to find a potential new user for the historical buildings which retain their *use*. The analysis of use, especially for the housing buildings, is fairly expensive. Such an example is former Women's Hospital of Bern, a building for which the use analysis yielded high potential for utilization, so the spatial organization is adapted according to the needs of the University of Bern, which thus obtained a significant extension in the vicinity of already existing premises.

The analysis of use for this example is very successfully conducted through two phases, which are separately realized:

Phase 1 included the current option of the owner to target the specific group of potential users through identification of potential use of the building. Therefore, the application of marketing in this phase is extremely important. Identification of the future use of derelict historical buildings is a systematic process, based on the open approach method, that is, the *brainstorm* creative method, where all the interested parties cooperate and exchange ideas. Within a creative workshop of kind, some parameters relating to the historical value of the building and action of various impacts on the building are observed. The impact of these parameters must be flexible, that is, they can be adapted within certain limits. A moderator must facilitate creation of a realistic picture



Shema 5. Predloženi pravci delovanja u procesu revitalizacije
 Scheme 5. Proposed courses for action in the revitalization process

Faza 2 nadovezuje se na fazu 1, ispitivanjem upotrebe objekta prvobitnog i zatečenog stanja i neophodnih mera prilagođavanja. Uspostavlja se veza između postojećih karakteristika zgrade i budućih zahteva korisnika.

Analiza namene ne može da zameni analizu tržišta i gradilišta. Sve tri analize moraju biti sprovedene i veoma su važne pri određivanju nove funkcije. Analiza namene je koristan dodatak analizama tržišta i lokacije za tzv. kuće - spomenike kulture. Prenosnom definicijom *Spomenik kao ambasada* (tumači se kao zastupnik vrednosti jednog vremena), mnogi sukobi između službe zaštite, investitora i budućeg korisnika, u pogledu tretmana zaštite spomenika kulture, mogu da se izbegnu. Problem nastaje kada administrativne vlasti koje se bave zaštitom ne žele da ulože dodatni trud da bi opravdale i potvrdile sopstvenu procenu da je objekat vredan spomenik kulture. To je neophodno obaviti da bi se mere kompenzacije mogu utvrditi od strane gradske vlasti, i to

of the historical characteristics (architectonic, structural, style) of the buildings for the participants. These parameters serve to well assess the buildings in their entirety, according to the analyses proscribed in the revitalization process.

Phase 2 is a continuation of Phase 1 through the investigation of the original and current building uses and the necessary adaptation measures. A connection between the existing building characteristics and future user requirements is established.

The use analysis cannot replace the analysis of the market and construction sites. All three must be conducted, and they are very important in determination of the new function. The use analysis is a valuable addition to the market and location analyses for the so-called "houses as cultural monuments". Using the definition "Monument as an embassy", (shall be construed as representative values of a time) many conflicts between the protection office, investors and future users in terms

onog njenog segmenta kojem je primarni cilj zaštita istorijskog jezgra grada. Kompenzacija, na primer, nedovoljnog prostora za novu namenu, može da obuhvati nadogradnju, adaptaciju unutrašnjosti bez većih rušenja pregradnih zidova, korišćenje podrumskog prostora, povećanje iskorišćenosti unutrašnjeg prostora objedinjavanjem u veće prostorne jedinice, izgradnju novog segmenta građevine koji se integriše u postojeći objekat i slično. Povećanje mera kompenzacije praćeno je utvrđivanjem neophodnih troškova koji povećavaju cenu revitalizacije, ali zato čine istorijski objekat atraktivnijim za tržište.

U celokupnom procesu revitalizacije napuštenih, neiskorišćenih, zaštićenih građevina u istorijskom jezgru Berna uočavaju se dva problema koja još nisu dovoljno razrešena. Prvi je nedovoljno jako uporište u naučnom pristupu zaštiti graditeljskog nasleđa, jer je uočljiva subjektivnost individualnih ocena i procena. Pošto su zahtevi korisnika uvek subjektivne prirode, doza arbitrarosti svakako postoji.

Analiza tržišta na kojem se nalaze i istorijski objekti nije stabilna. Razlog jeste to što je švajcarsko tržište veoma liberalizovano i izuzetno atraktivno zbog velike kupovne moći stanovništva. Bez obzira na to da li je reč o kupovini istorijskog objekta ili samo o njegovom zakupu, mogućnost revitalizacije ili čak i rekonstrukcije, dostupna je kako pojedincima, tako i raznim pravnim licima, na primer, velikim robnim lancima, što najčešće podrazumeva obuhvat nekoliko povezanih istorijskih zgrada i predstavlja složen i zahtevan poduhvat.

Preispitivanje finansijskog opterećenja svih etapa revitalizacije nije bilo predmet ovog rada, s obzirom na to što zavisi od niza faktora, pre svega od veličine samog istorijskog objekata, a zatim i od obima intervencija u procesu revitalizacije.

Ovaj uspešni metod određivanja potencijala za revitalizaciju graditeljskog nasleđa, kao švajcarsko iskustvo, može naći svoju primenu u Srbiji, budući da ekonomska, odnosno tržišna vrednost graditeljskog nasleđa ovde još uvek nije prihvaćena kao osnova velikog potencijala koje nasleđe može da ugradi u razvoj privrede i kulture društva. Da bi se to ostvarilo, pored razvijanja društvene svesti o koristi takvog pristupa, neophodna je i izmena ili dopuna čitavog niza zakona, koji u ovom obliku nisu kompatibilni i ne mogu da podrže navedene aktivnosti. Prvi korak bi trebalo da bude usklađivanje odavno prevaziđenih, zastarelih zakona u pogledu zaštite kulturnih dobara, sa zakonodavstvom Evropske zajednice. Švajcarska država je svoje zakonodavstvo prvo uskladila s međunarodnim zakonodavstvom, pa je tek onda počela da razvija navedenu strategiju revitalizacije.

Najveći izazov s kojim se susreću svi akteri prilikom revitalizacije jeste to da svojim umećem doprinesu da kvalitetno odabrana nova namena, putem kreativno osmišljene ideje revitalizacije, doprinese produženju životnog veka istorijske građevine i da unapredi njene već utvrđene vrednosti.

of treatment of the protection status of the cultural monument can be avoided. The problem occurs when administrative authorities engaged in protection do not wish to put more effort in justifying and confirming their own assessment that a building is a valuable cultural monument. This is necessary to accomplish in order to determine the compensation measures by the city authorities, particularly the segment dealing with the primary goal of protection of historical core of the town. The compensation, for instance, of the insufficient space for the new use can include extension, interior adaptation without demolishing of partition walls, usage of basement space, and increase of utilization of interior space by unifying it into large spatial units, construction a new segment of the building, which is integrated in the existing building etc. The increase of the compensation measures is followed by determination of the necessary costs increasing the revitalization costs, but which render the historical building more attractive for the market.

The entire process of revitalization of abandoned, disused, listed buildings in the historical core of Bern features two problems which have not been properly resolved. The first problem is insufficiently strong scientific foundations of the approach to protection of the building heritage, because there are a lot of individual assessments and evaluations. Since the demands of the users are always subjective in nature, some arbitrariness is surely present.

The analysis of the market which includes historical buildings is unstable. The reason for that is that the Swiss market is much liberalized and extremely attractive because of the high economic power of the population. Regardless of whether a historical building is purchased or rented, the potential for revitalization or even reconstruction is available both to the individuals and various legal entities, for instance large department store chains, which most frequently comprises inclusion of several adjacent historical buildings, and represents a complex and demanding undertaking.

Analysis of the financial load of all the stages of revitalization was not the subject of this paper, considering that it depends on a number of factors, primarily on the size of the historical building itself, and on the scope of interventions in the revitalization process.

Yet, the method of determination of potential for revitalization of historical heritage, as a Swiss experience, can be implemented in Serbia, regarding that the economy, that is, market value of the built heritage has not been accepted locally as basis of a high potential for development of the economy and culture of the society. In order to accomplish that, in addition to the development of the social awareness of the benefits of such approach, it is necessary to change or amend a number of regulations which are not compatible with these goals and cannot support the mentioned activities. The first step should be the harmonization of the Serbian (outdated) legal regulations referring to the protection of the cultural assets with the EU regulations. The Swiss state firstly harmonized their regulations with the international ones, and only then started to develop the mentioned revitalization strategy.

The biggest challenge encountered by all the participants in the revitalization process is how to implement their skill to contribute so that the well-chosen new use

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REZIME

METODE ODREĐIVANJA POTENCIJALA ZA REVITALIZACIJU GRADITELJSKOG NASLEĐA – POUKE GRADA BERNA

Goran STANIŠIĆ
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Srpski konzervatori mogu naučiti mnogo iz primera revitalizacije Starog grada Berna. Analize pokazuju da ovaj primer načina revitalizacije i zaštite napuštenih, nenaseljenih spomenika kulture zavisi od njihovog budućeg korišćenja. Analiza buduće namene nasleđa nije dovoljna da se koristiti kao postupak za njegovu revitalizaciju. Primena novih ideja na osnovu istorijskog značaja zaštićenog objekta generiše njegovu strukturu. Različitim analizama testiran je pristup revitalizaciji putem strukturiranja spomenika kulture i njihovog prilagođavanja za novu namenu. Utvrđeno je da se buduća namena spomenika kulture ne može utvrditi samo definisanjem metoda revitalizacije koji je odredio neki javni organ, već da budući vlasnik zgrade mora aktivno učestvovati u svemu tome. Daje su i preporuke za prenos ove metodologije pri rešavanju problema srpskog graditeljskog nasleđa.

Ključne reči: metode revitalizacija, graditeljsko nasleđe, ekonomski potencijal, funkcionalna iskorišćenost, grad Bern.

SUMMARY

METHODS FOR DETERMINATION OF REVITALIZATION POTENTIAL OF BUILT HERITAGE – LESSONS LEARNED ON THE CITY OF BERN

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Serbian conservationists could learn a lot from the revitalization example of the Old city of Bern. Analyses of this example show that the method of revitalization and protection of listed, but abandoned and inhabited, buildings depends on its future use. Analyses of future purpose of built heritage cannot be used as a method for its revitalization. The use of new ideas based on historical importance of the protected building generates the building's structures. The approach of revitalization through the process of building structuring and adaptation to new purpose has been tested through different analyses. It was found that the future purpose of building cannot be determined only by defining the revitalization method, set by public body, but in this process the future owner of building should actively participate. The recommendations for transferring this methodology in solving Serbian built heritage problems are given.

Key words: methods of revitalization, architectural heritage, economic potential, functional utilization, city of Bern.

PRIMENA SLOJEVITIH KONAČNIH ELEMENATA U NUMERIČKOJ ANALIZI LAMINATNIH KOMPOZITNIH I SENDVIČ-PLOČA I LJUSKI S DELAMINACIJAMA

APPLICATION OF LAYERED FINITE ELEMENTS IN THE NUMERICAL ANALYSIS OF LAMINATED COMPOSITE AND SANDWICH STRUCTURES WITH DELAMINATIONS

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1 UVOD

Laminatni kompoziti su moderni materijali koji se široko primenjuju u različitim granama industrije, a najviše u mašinstvu i građevinarstvu. Ovi materijali, kao što su ugljenična vlakna, staklena vlakna ili polimeri ojačani vlaknima, imaju izuzetno visoku čvrstoću i krutost, uz relativnu malu sopstvenu težinu. Na primer, konstrukcije koje se primenjuju u avio i svemirskoj industriji izgrađene su od tankozidnih kompozitnih cilindričnih ili sferičnih delova [1], koji imaju odličnu otpornost na zamor ili koroziju. Zbog velikog potencijala za različite primene, laminatni kompoziti neprekidno privlače pažnju mnogih istraživača [2–4]. Drugi tip materijala koji se mogu analizirati primenom predloženog numeričkog modela jesu sendvič-paneli s mekim jezgrom, koji se zbog male težine primenjuju u građevinarstvu u vidu lakih krovnih i fasadnih termoizolacionih panela.

Osnovni faktor koji skraćuje životni vek laminatnih kompozitnih konstrukcija jeste prisustvo delaminacije koja nastaje kao posledica grešaka pri spajanju lamina u fabričkoj proizvodnji. U slučaju sendvič-panela veoma je bitno da idealna veza između obloge panela i mekog jezgra ne bude narušena, kako bi se panel ponašao u skladu s projektovanim zahtevima. Autori su ranije pokazali da prisustvo delaminacije ozbiljno utiče na mehaničke karakteristike laminatnih kompozitnih i sendvič-ploča [5–6].

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1 INTRODUCTION

Laminar composites are modern engineering materials applied widely in different industries, mostly in the mechanical and civil engineering. These materials, such as carbon-fiber, glass-fiber or fiber-reinforced polymers allow for high strength and stiffness at a relatively low weight. For example, the aerospace structures are generally made of thin-walled composite cylindrical or spherical shell components [1], which have an excellent fatigue and corrosion resistance properties. Because of a great potential for the structural applications, laminar composites continuously attract the attention of many researchers [2, 3, 4]. Another type of plate structures relevant for the considered numerical model is a soft-core sandwich panel, which low weight property makes them applicable in civil engineering as light roof and wall panels to provide the thermal isolation of buildings.

A major limiting factor for the lifetime of laminated composite structures is the presence of embedded delamination, resulting from the different fabrication-induced faults in the joining of laminas. In the case of sandwich panels it is important that the perfect bond between the face sheets and the soft-core remain intact for the panel to perform on the designed level. Authors have already shown that the presence of embedded delamination seriously influences the mechanical properties of laminated composite and sandwich plates [5, 6].

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linearne funkcije z-koordinate, kontinualne od sloja do sloja, kojima se interpoliraju komponente pomeranja u ravni. $H^l(z)$ su Heaviside-ove funkcije koje opisuju kinematiku delaminacije u l -tom sloju gde postoji oštećenje [5, 6, 8, 29]. Predloženim modelom ploče moguće je razmatrati proizvoljan broj delaminacija primenom odgovarajućeg broja dodatnih funkcija $H^l(z)$ u polju pomeranja.

2.3 Kinematičke relacije

Kinematičke relacije kojima se pretpostavljaju male dilatacije i umereno male rotacije u skladu sa Von Kármán-ovim pretpostavkama definišu polje deformacija, koje se može podeliti na linearan (L) i geometrijski nelinearan (NL) deo:

$$\begin{aligned}\varepsilon_x^L &= \frac{\partial u}{\partial x} + \sum_{l=1}^N \frac{\partial u^l}{\partial x} \Phi^l + \sum_{l=1}^{ND} \frac{\partial U^l}{\partial x} H^l \\ \varepsilon_y^L &= \frac{\partial v}{\partial y} + \sum_{l=1}^N \frac{\partial v^l}{\partial y} \Phi^l + \sum_{l=1}^{ND} \frac{\partial V^l}{\partial y} H^l \\ \gamma_{xy}^L &= \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} + \sum_{l=1}^N \left(\frac{\partial u^l}{\partial y} + \frac{\partial v^l}{\partial x} \right) \Phi^l + \sum_{l=1}^{ND} \left(\frac{\partial U^l}{\partial y} + \frac{\partial V^l}{\partial x} \right) H^l \\ \gamma_{xz}^L &= \frac{\partial w}{\partial x} + \sum_{l=1}^N u^l \frac{d\Phi^l}{dz} + \sum_{l=1}^{ND} \frac{\partial W^l}{\partial x} H^l \\ \gamma_{yz}^L &= \frac{\partial w}{\partial y} + \sum_{l=1}^N v^l \frac{d\Phi^l}{dz} + \sum_{l=1}^{ND} \frac{\partial W^l}{\partial y} H^l\end{aligned}\quad (2)$$

$$\begin{aligned}\varepsilon_x^{NL} &= \frac{1}{2} \left(\frac{\partial w}{\partial x} \right)^2 + \frac{\partial w}{\partial x} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial x} H^l + \frac{1}{2} \sum_{l=1}^{ND} \sum_{j=1}^{ND} \frac{\partial W^l}{\partial x} \frac{\partial W^j}{\partial x} H^l H^j \\ \varepsilon_y^{NL} &= \frac{1}{2} \left(\frac{\partial w}{\partial y} \right)^2 + \frac{\partial w}{\partial y} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial y} H^l + \frac{1}{2} \sum_{l=1}^{ND} \sum_{j=1}^{ND} \frac{\partial W^l}{\partial y} \frac{\partial W^j}{\partial y} H^l H^j \\ \gamma_{xy}^{NL} &= \frac{\partial w}{\partial x} \frac{\partial w}{\partial y} + \sum_{l=1}^{ND} \sum_{j=1}^{ND} \frac{\partial W^l}{\partial x} \frac{\partial W^j}{\partial y} H^l H^j + \frac{\partial w}{\partial x} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial y} H^l + \frac{\partial w}{\partial y} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial x} H^l \\ \gamma_{xz}^{NL} &= \gamma_{yz}^{NL} = 0\end{aligned}\quad (3)$$

2.4 Konstitutivne relacije pojedinačnog sloja

U ovom radu je usvojen konstitutivni model koji pretpostavlja ravno stanje napona u ploči, jer je u slučaju tankih ploča 3D konstitutivni model numerički nestabilan [5, 6]. Uz pretpostavku Hooke-ovog zakona linearne elastičnosti, konstitutivne relacije k -tog sloja za ravno stanje napona, u globalnom koordinatnom sistemu, mogu se prikazati kao:

boundary conditions $U^l=V^l=W^l=0$ on the l^{th} crack boundary. $\Phi^l(z)$ are linear layerwise continuous functions of the z -coordinate for interpolation of in-plane displacement components. H^l are Heaviside step functions to describe the delamination kinematics in l^{th} delaminated layer [5, 6, 8, 29]. The proposed plate model allows for the consideration of an arbitrary number of delaminations by using an arbitrary number of additional delamination expansions in the displacement field. In-plane displacements are piece-wise continuous through the thickness of the laminate in the intact region with discontinuities at the delaminated interfaces.

2.3 Kinematic relations

Kinematic relations assuming the small strains and moderately large rotations according to von Kármán's assumptions define the strain field which can be divided into a linear (L) and geometrically nonlinear (NL) part:

2.4 Constitutive relations of the individual layer

A plane stress constitutive model is adopted because thin plates 3D constitutive model suffer the numerical instabilities [5, 6]. The constitutive equations of the k^{th} orthotropic lamina for the plane stress state that follows linear elastic Hooke's law, in the global coordinate system, can be written as:

$$\{\sigma\}^{(k)} = [\bar{Q}]^{(k)} \{\varepsilon\}^{(k)} \quad (4)$$

U jednačini (4), $[\bar{Q}]^{(k)}$ je redukovana matrica krutosti k -tog sloja u globalnom koordinatnom sistemu, dobijena iz matrice jednačine ($[T]^{(k)}$ -matrica transformacije):

$$[\bar{Q}]^{(k)} = [T]^{(k)-1} [Q]^{(k)} [T]^{(k)} \quad (5)$$

In Eq. (4), $[\bar{Q}]^{(k)}$ is the matrix of reduced stiffness components of the k^{th} lamina in the global coordinate system, derived using the matrix relation ($[T]^{(k)}$ -transformation matrix):

2.5 Princip virtualnog rada

Princip virtualnog rada za dinamički opterećene konstrukcije u vremenskom intervalu $[0, T]$ izveden je primenom Hamilton-ovog principa:

$$\int_0^T \left\{ \int_V [\sigma_{ij} \delta \varepsilon_j - q(x, y, t) - \rho(\ddot{u}_1 \delta u_1 + \ddot{u}_2 \delta u_2 + \ddot{u}_3 \delta u_3)] dV \right\} dt = 0 \quad (6)$$

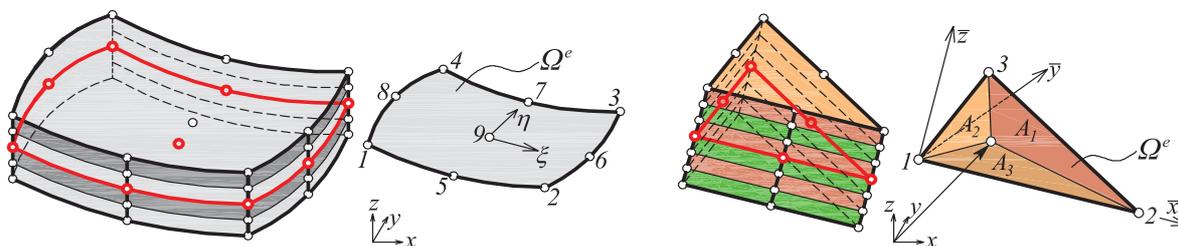
U jednačini (6) q predstavlja poprečno raspodeljeno opterećenje u srednjoj ravni ploče, $\ddot{\mathbf{u}}$ je vektor ubrzanja, ρ je gustina i V je zapremina razmatranog domena. Ukoliko uvedemo presečne sile kao integrale komponentalnih napona po visini ploče dobijamo princip virtualnog rada u sledećem obliku:

$$\int_0^T (\delta U^L + \delta U^{NL} + \delta V - \delta K) dt = 0 \quad (7a)$$

$$\delta U^L = \int_{\Omega} \left\{ \begin{aligned} & N_x \frac{\partial \delta u}{\partial x} + N_y \frac{\partial \delta v}{\partial y} + N_{xy} \left(\frac{\partial \delta u}{\partial y} + \frac{\partial \delta v}{\partial x} \right) + Q_x \frac{\partial \delta w}{\partial x} + Q_y \frac{\partial \delta w}{\partial y} + \\ & \sum_{l=1}^N \left(N'_x \frac{\partial \delta u^l}{\partial x} + N'_y \frac{\partial \delta v^l}{\partial y} + N'_{xy} \left(\frac{\partial \delta u^l}{\partial y} + \frac{\partial \delta v^l}{\partial x} \right) + Q'_x \delta u^l + Q'_y \delta v^l \right) + \\ & \sum_{l=1}^{ND} \left(\bar{N}'_x \frac{\partial \delta U^l}{\partial x} + \bar{N}'_y \frac{\partial \delta V^l}{\partial y} + \bar{N}'_{xy} \left(\frac{\partial \delta U^l}{\partial y} + \frac{\partial \delta V^l}{\partial x} \right) + \bar{Q}'_x \frac{\partial \delta W^l}{\partial x} + \bar{Q}'_y \frac{\partial \delta W^l}{\partial y} \right) \end{aligned} \right\} d\Omega \quad (7b)$$

$$\delta U^{NL} = \int_{\Omega} \left\{ \begin{aligned} & N_x \frac{\partial w}{\partial x} \frac{\partial \delta w}{\partial x} + N_y \frac{\partial w}{\partial y} \frac{\partial \delta w}{\partial y} + N_{xy} \left(\frac{\partial w}{\partial y} \frac{\partial \delta w}{\partial x} + \frac{\partial w}{\partial x} \frac{\partial \delta w}{\partial y} \right) + \\ & \bar{N}'_x \left(\frac{\partial \delta w}{\partial x} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial x} + \frac{\partial w}{\partial x} \sum_{l=1}^{ND} \frac{\partial \delta W^l}{\partial x} \right) + \bar{N}'_y \left(\frac{\partial \delta w}{\partial y} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial y} + \frac{\partial w}{\partial y} \sum_{l=1}^{ND} \frac{\partial \delta W^l}{\partial y} \right) + \\ & \bar{N}'_{xy} \left(\frac{\partial \delta w}{\partial x} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial y} + \frac{\partial w}{\partial x} \sum_{l=1}^{ND} \frac{\partial \delta W^l}{\partial y} + \frac{\partial \delta w}{\partial y} \sum_{l=1}^{ND} \frac{\partial W^l}{\partial x} + \frac{\partial w}{\partial y} \sum_{l=1}^{ND} \frac{\partial \delta W^l}{\partial x} \right) + \\ & \bar{N}'_x \sum_{l,j=1}^{ND} \frac{1}{2} \left(\frac{\partial W^j}{\partial x} \frac{\partial \delta W^l}{\partial x} + \frac{\partial W^l}{\partial x} \frac{\partial \delta W^j}{\partial x} \right) + \bar{N}'_y \sum_{l,j=1}^{ND} \frac{1}{2} \left(\frac{\partial W^j}{\partial y} \frac{\partial \delta W^l}{\partial y} + \frac{\partial W^l}{\partial y} \frac{\partial \delta W^j}{\partial y} \right) + \\ & \bar{N}'_{xy} \sum_{l,j=1}^{ND} \left(\frac{\partial W^j}{\partial y} \frac{\partial \delta W^l}{\partial x} + \frac{\partial W^l}{\partial x} \frac{\partial \delta W^j}{\partial y} \right) \end{aligned} \right\} d\Omega \quad (7c)$$

$$\sum_{i=1}^3 A_i = \sum_{i=1}^3 \xi_i A = A \quad \sum_{i=1}^3 \xi_i = \xi_1 + \xi_2 + \xi_3 = 1 \quad \xi_3 = 1 - \xi_1 - \xi_2 \quad (8)$$



Slika 2. Lagrange-ov četvorougaoni konačni element sa devet čvorova (levo) i trougaoni slojeviti konačni elementi sa tri čvora (desno) u prirodnom $(\xi-\eta)$ i globalnom (xyz) koordinatnom sistemu ($\bar{X}\bar{Y}\bar{Z}$ - lokalni koordinatni sistem trougaonog elementa)

Figure 2. Lagrange 9-node quadrilateral (left) and 3-node triangular (right) layered finite elements in natural $(\xi-\eta)$ and global (xyz) coordinate systems ($\bar{X}\bar{Y}\bar{Z}$ - local coordinate system of the triangular element)

Mreža konačnih elemenata generiše se u 2D ravni, a usvojene interpolacione funkcije po visini ploče se koriste za interpolaciju nepoznatih upravno na ravan laminata (čime se iz proračuna eliminiše z-koordinata). Ova pretpostavka omogućava da se interpolacija nepoznatih u ravni i upravno na ravan laminata vrši nezavisno. Nepoznate komponente pomeranja interpoliraju se u lokalnoj ravni pojedinačnog konačnog elementa, definisanog s tri ugla trougla, dok lokalna x-osa povezuje čvorove 1–2. Lokalna y-osa nalazi se u ravni elementa i upravna je na x-osu, dok je lokalna z-osa upravna na ravan elementa i formira Dekartov koordinatni sistem $(\bar{X}\bar{Y}\bar{Z})$ lociran u čvoru 1. Radi jednostavnosti, za sva generalisana pomeranja koriste se iste interpolacione funkcije:

The FE mesh is generated in the 2D plane, and the adopted interpolation functions through the plate thickness are used for out-of-plane interpolation of the unknown variables (which eliminates the z-coordinate from the calculation). This assumption allows interpolating the unknown field variables independently for the in-plane and out-of-plane distribution. The unknown displacement components are interpolated in the local plane of the single finite element, defined by three corner nodes, while the local x-axis connects nodes 1-2. The local y-axis is positioned in the element plane and it is perpendicular to the local x-axis, while the local z-axis is perpendicular to the element plane and forms the local Cartesian orthogonal coordinate system $(\bar{X}\bar{Y}\bar{Z})$, located in the node 1. For the sake of simplicity, the same interpolation functions are used for the interpolation of all generalized displacements:

$$\begin{Bmatrix} u \\ v \\ w \end{Bmatrix} = \begin{Bmatrix} \sum_{i=1}^m u_i \psi_i \\ \sum_{i=1}^m v_i \psi_i \\ \sum_{i=1}^m w_i \psi_i \end{Bmatrix} = [\Psi] \{\Delta\}, \quad \begin{Bmatrix} u' \\ v' \end{Bmatrix} = \begin{Bmatrix} \sum_{i=1}^m u'_i \psi_i \\ \sum_{i=1}^m v'_i \psi_i \end{Bmatrix} = [\bar{\Psi}] \{\Delta^l\}, \quad \begin{Bmatrix} U' \\ V' \\ W' \end{Bmatrix} = \begin{Bmatrix} \sum_{i=1}^m U'_i \psi_i \\ \sum_{i=1}^m V'_i \psi_i \\ \sum_{i=1}^m W'_i \psi_i \end{Bmatrix} = [\Psi] \{\bar{\Delta}^l\} \quad (9)$$

U jednačinama (9), $\{\Delta\}$, $\{\Delta^l\}$ i $\{\bar{\Delta}^l\}$ predstavljaju vektore pomeranja u srednjoj ravni, l -tom numeričkom sloju l -tom numeričkom sloju u kome postoji delaminacija, respektivno. Indeks m označava broj čvorova konačnog elementa. Primenjeni su pravougaoni Lagrange-ovi konačni elementi sa četiri ili devet čvorova, kao i trougaoni slojeviti konačni elementi s tri čvora. $[\Psi]$ i $[\bar{\Psi}]$ su matrice Lagrange-ovih interpolacionih funkcija:

In Eqs. (9), $\{\Delta\}$, $\{\Delta^l\}$ and $\{\bar{\Delta}^l\}$ denote displacement vectors in the middle plane, the l^{th} numerical layer and the l^{th} delaminated layer, respectively. Index m denotes the number of nodes per element. Four- and nine-node Lagrange quadrilateral, as well as three-node triangular layered finite elements are derived. $[\Psi]$ and $[\bar{\Psi}]$ are matrices of Lagrangian interpolation functions:

$$[\Psi] = \begin{bmatrix} \psi_1 & 0 & 0 & \dots \\ 0 & \psi_1 & 0 & \dots \\ 0 & 0 & \psi_1 & \dots \\ \vdots & \vdots & \vdots & \ddots \end{bmatrix}_{3 \times 3m}, \quad [\bar{\Psi}] = \begin{bmatrix} \psi_1 & 0 & \dots \\ 0 & \psi_1 & \dots \\ \vdots & \vdots & \ddots \end{bmatrix}_{2 \times 2m} \quad (10)$$

3.2 Jednačine kretanja

Zamenom interpolacije komponentata pomeranja iz jednačine (9) u princip virtualnih pomeranja (jednačine (7)), dobijamo potpuno diskretizovan numerički model MKE [5, 6, 29]. Na ovaj način dobijamo sistem jednačina kretanja na nivou konstrukcije:

$$[M]\{\ddot{d}\} + [C]\{\dot{d}\} + [K^L + K^{NL}]\{d\} = \{F\} \quad (11)$$

Matrica masa $[M]$, matrica prigušenja $[C]$, linearne i nelinearne matrice krutosti $[K^L]$ i $[K^{NL}]$ i globalni vektor sila $\{F\}$ dobijaju se sabiranjem odgovarajućih članova u karakterističnim matricama i vektorima pojedinačnih elemenata.

Sve matrice dobijene su primenom Gauss-Legendre-ove integracije na domenu pojedinačnog konačnog elementa, označenog sa Ω^e . Kako bi se iz proračuna eliminisala nepostojeća smičuća krutost (fenomen „shear locking, primenjena je selektivna integracija. Gauss-Legendre-ova integracija na trougaonom domenu sračunava se prema izrazu:

$$\int_{\Omega^e} F(\bar{x}, \bar{y}) d\Omega^e = \int_0^{1-\xi_2} \int_0^{\xi_2} F(\xi, \eta) d\xi d\eta \cdot \det[J] = \sum_{i=1}^{n_p} W_i f(\xi^i, \eta^i) \quad (12)$$

U jednačini (12), F je funkcija koju treba numerički sračunati, $[J]$ je Jacobi-eva matrica, n_p je broj integracionih tačaka, ξ^i, η^i su koordinate i -te integracione tačke i W_i je odgovarajući težinski koeficijent [43]. Submatrice matrica krutosti i masa pojedinačnog konačnog elementa date su u [29]. Sledeće jednačine opisuju dva problema koji su razmatrani u ovom radu:

1. Linearne slobodne vibracije $([K^L] - \omega^2[M])\{d\} = 0$
2. Geometrijski nelinearna dinamička analiza

$$[M]\{\ddot{d}\} + [C]\{\dot{d}\} + [K^L + K^{NL}]\{d\} = \{F\}$$

3.3 Transformacija u globalne koordinate i formiranje matrica sistema

Kada se proizvoljna ljuska deli na trougaone elemente, svaki element ima proizvoljnu orijentaciju u globalnom koordinatnom sistemu, pa je iz tog razloga od značaja da pogodno definišemo matrice transformacije svakog konačnog elementa – potrebno je definisati kosinuse uglova koje lokalne ose zaklapaju s globalnim osama. Uzimajući u obzir ranija razmatranja, broj čvornih stepeni slobode u lokalnom koordinatnom sistemu je: $n_{DOF,LOCAL} = 3+2 \times N+3 \times ND$. Ove lokalne komponente pomeranja transformišu se primenom matrice transformacije konačnog elementa $[\hat{T}]$ u globalne komponente

3.2 Equations of motion

When substituting the interpolation of displacement components from Eq. (9) into the principle of virtual displacements (Eqs. (7)), the fully discretized finite element model [5, 6, 29] has been obtained. This leads to the system of equations of motion on the structural level:

The mass matrix $[M]$, the damping matrix $[C]$, the linear and nonlinear structural stiffness matrices $[K^L]$ and $[K^{NL}]$ and the global force vector $\{F\}$ are obtained from the assembly of the respective element matrices and element load vector.

All element matrices are derived using Gauss-Legendre quadrature over single finite element domain, denoted as Ω^e . Selective integration is used for the elimination of spurious shear stiffness from calculation (shear locking phenomenon). The Gauss-Legendre quadrature for triangular domain is written as:

In Eq. (12), F is the function to be calculated numerically, $[J]$ is the Jacobi matrix, n_p is the number of integration points, ξ^i, η^i are the coordinates of the i^{th} integration point and W_i is the corresponding weighting factor [43]. The submatrices of the element stiffness and mass matrices are derived in [29]. Two problems investigated in this paper are described using the following equations:

1. Linear free vibrations $([K^L] - \omega^2[M])\{d\} = 0$
2. Geometrically nonlinear transient analysis

$$[M]\{\ddot{d}\} + [C]\{\dot{d}\} + [K^L + K^{NL}]\{d\} = \{F\}$$

3.3 Transformation to global coordinates and assembly procedure

When the arbitrary shell is divided into triangular elements, each element has an arbitrary orientation in the global coordinate system, so it is now important to conveniently define the transformation matrices for each element - we need to define the cosines of each finite element. Following the preceding considerations, the number of nodal degrees of freedom in the local coordinate system is: $n_{DOF,LOCAL} = 3+2 \times N+3 \times ND$. These local displacement components are transformed using the element transformation matrix $[\hat{T}]$ into the global

pomeranja - čvorne stepene slobode u globalnom koordinatnom sistemu $n_{DOF,GLOBAL} = 3+3 \times N + 3 \times ND$. Matrice krutosti ili masa pojedinačnog konačnog elementa u globalnom koordinatnom sistemu sračunavaju se kao:

$$\begin{aligned} [K]^e &= [\hat{T}]^T [K] [\hat{T}] \\ [M]^e &= [\hat{T}]^T [M] [\hat{T}] \end{aligned} \quad (13)$$

4 REŠENJE VREMENSKOG PROBLEMA I KONTAKTNI ALGORITAM

Za integraciju u vremenu je primenjena Newmark-ova implicitna integraciona šema [8]. Ubrzanja i brzine su aproksimirani primenom redukovanih Taylor-ovih redova. Pomeranja i brzine određeni su primenom rekurentnih formula [8], a početni granični uslovi su homogeni. S obzirom na to što je matrica $[K^{NL}]$ funkcija nepoznatog pomeranja $\{d\}_{n+1}$, sistem uslovnih jednačina sistema mora se rešavati iterativno sve dok kriterijum konvergencije ne bude zadovoljen. Picard-ov metod [8] primenjuje se sve dok greška ne bude manja ili jednaka vrednosti neke unapred usvojene tolerancije (recimo $\varepsilon \leq 1\%$). Prigušenje je zanemareno.

Tokom dinamičkog odgovora laminatnih kompozitnih ili sendvič-ploča s delaminacijom, može se formirati mali međuprostor između susednih slojeva u zoni delaminacije. Nakon toga razdvojeni slojevi tokom kretanja mogu ponovo da dođu u kontakt i na taj način zatvore postojeći međuprostor. Ovaj fenomen se u literaturi naziva „disanje“ delaminacije [36–37], koje se može modelirati primenom kontaktnih uslova između čvorova, bez uzimanja u obzir trenja [6]. Kontaktни algoritam uspešno sprečava međusobno prodiranje sloja u sloj tokom dinamičkog odgovora kompozitnih ploča s delaminacijom. Ovo važi i za linearnu i za geometrijski nelinearnu analizu.

5 NUMERIČKI PRIMERI I DISKUSIJA

U ranijim radovima [5, 6, 15, 30] autori su dokazali da ESL teorije precenjuju vrednosti sopstvenih frekvencija i potcenjuju vrednosti ugiba kod pravougaonih kompozitnih i sendvič-ploča bez oštećenja. U ovom radu su proširene mogućnosti primene modela na analizu slobodnih vibracija kružnih kompozitnih ploča s delaminacijama.

5.1 Analiza linearnih slobodnih vibracija

Primer 5.1.1. U prvom primeru [44] razmatra se neoštećena ukleštena (CC) četvoroslojna kružna kompozitna ploča simetrične $(\theta/-\theta/-\theta/\theta)$ šeme laminacije. Prečnik ploče označen je sa a , a ukupna debljina ploče sa h . Svi slojevi su jednake debljine h_k . Za sve slojeve su pretpostavljeni ortotropni konstitutivni modeli sa sledećim materijalnim karakteristikama: $E_1/E_2 = 40$, $G_{12}/E_2 = G_{13}/E_2 = 0.6$, $G_{23}/E_2 = 0.5$, $\nu_{12} = \nu_{13} = \nu_{23} = 0.25$, $\rho = \text{const}$. Granični uslovi su zadati na ukleštenim

displacement components – nodal degrees of freedom in global coordinate system $n_{DOF,GLOBAL} = 3+3 \times N + 3 \times ND$. The global stiffness/mass matrices of the triangular finite element can be calculated as follows:

4 SOLUTION OF THE TIME DEPENDENT PROBLEM AND CONTACT ALGORITHM

For integration in time, an implicit Newmark's integration scheme is employed [8]. The accelerations and velocities are approximated using truncated Taylor's series. Displacements and velocities are approximated using recursive formulae [8], while the homogenous initial conditions are prescribed. The assembled equation must be solved iteratively until the convergence criterion is satisfied since the matrix $[K^{NL}]$ is the function of displacements $\{d\}_{n+1}$. The Picard method [8] is employed until the error is less than or equal to some prescribed tolerance (say $\varepsilon \leq 1\%$). The structural damping is neglected.

During the transient response of delaminated composite or sandwich structures, a small gap may be formed between the adjacent layers in delaminated zones of the plate. After that the separated layers may unload and again contact each other at that delaminated interface. This phenomenon is referred to as "breathing" of a delamination [36-37], which can be modelled using the node-to-node frictionless contact conditions [6]. The contact algorithm successfully "corrects" the interlaminar penetration during the transient response of the delaminated composite plate both in the linear and geometrically nonlinear analysis.

5 NUMERICAL EXAMPLES AND DISCUSSION

In previous works [5, 6, 15, 30], authors have proven that ESL theories overestimate the fundamental frequencies and underestimate the transverse deflections of rectangular intact composite and sandwich plates. The applicability of the model is extended here for the free vibrations analysis of circular composite plates with delaminations.

5.1 Linear free vibrations analysis

Example 5.1.1. The first benchmark example [44] is concerned with an intact 4-layer clamped (CC) circular composite plate with symmetric $(\theta/-\theta/-\theta/\theta)$ stacking sequence. Plate diameter is denoted as a , while the overall plate thickness is denoted as h . All laminas are of equal thickness h_k . The following material parameters are assumed for orthotropic constitutive models of all laminas: $E_1/E_2 = 40$, $G_{12}/E_2 = G_{13}/E_2 = 0.6$, $G_{23}/E_2 = 0.5$, $\nu_{12} = \nu_{13} = \nu_{23} = 0.25$, $\rho = \text{const}$. The boundary conditions

ivicama sprečavanjem svih generalisanih pomeranja u čvorovima. U ovom proračunu su zanemareni članovi koji odgovaraju velikim rotacijama. Rezultati su dobijeni primenom nestrukturirane mreže sa 931 slojevitim konačnim elementom sa devet čvorova, s redukovanom integracijom (broj čvorova je 3849). Sračunate su bezdimenzionalne sopstvene frekvencije neoštećenih kružnih laminatnih kompozitnih ploča prema izrazu $\Omega = \omega \cdot a^2/h(\rho/E_2)^{1/2}$ i izvršeno je poređenje s rezultatima dobijenim primenom različitih ESL teorija: smičuće teorije ploča [44] i smičuće teorije ploča prvog reda [45]. Rezultati su objedinjeni u Tabeli 1.

are prescribed along clamped boundaries by constraining all generalized displacements in edge nodes. The terms related to the large rotations in the kinematic equations are omitted in this calculation. The results are obtained using the unstructured FE mesh of 931 9-node layered elements with reduced integration (number of nodes is 3849). The nondimensionalized fundamental frequencies $\Omega = \omega a^2/h(\rho/E_2)^{1/2}$ of intact circular laminated composite plates is calculated and compared with results obtained using different ESL theories: transverse shear deformation theory [44] and First-Order Shear Deformation Theory [45]. The results are elaborated in Table 1.

Tabela 1. Bezdimenzionalne sopstvene frekvencije četvoroslojne uklještene kružne kompozitne ploče simetrične angle-ply (θ - θ - θ / θ) šeme laminacije

Table 1. The nondimensionalized fundamental frequencies of the 4-layer clamped circular laminated composite plates with symmetric (θ - θ - θ / θ) angle-ply stacking sequence

Numerical model	$\theta = 0$	$\theta = 30$	$\theta = 45$
SDT [36]	23.130	24.063	24.557
FSDT [37]	22.211	24.071	24.752
GLPT, Present	22.913	25.107	25.684

Na osnovu Tabele 1 očigledno je da je predloženi model u stanju da predvidi sopstvene frekvencije kružnih laminatnih kompozitnih ploča, čak i uz korišćenje četvorougaoih elemenata za modeliranje kružne geometrije. Za sve šeme laminacije dobijen je nešto krući odgovor (2–4%). Sopstvene frekvencije očigledno rastu s povećanjem nivoa ortotropije.

Primer 5.1.2. U drugom primeru [13] razmatra se kvadratna kompozitna ploča sa osam slojeva, simetrične (0/90/45/90)_s šeme laminacije. Ivice ploče imaju dužinu $a = 250\text{mm}$, dok je ukupna visina ploče $h = 2.12\text{mm}$. Svi slojevi su jednake debljine. Za ortotropni konstitutivni model svih slojeva pretpostavljeni su sledeći parametri materiala: $E_1 = 132\text{GPa}$, $E_2 = 5.35\text{GPa}$, $G_{12} = G_{13} = 2.79\text{GPa}$, $\nu_{12} = \nu_{13} = 0.291$, $\nu_{23} = 0.300$, $\rho = 1446.2\text{kg/m}^3$. Kvadratna delaminacija, stranice $a_{del} = a/2$ ranije je ubačena u srednjoj ravni (između slojeva 4 i 5), u centru ploče. Granični uslovi zadati su duž ivica ploče na sledeći način:

- slobodno oslonjene (SS_x) ivice: za $x = 0$ i $x = a$: $v = w = v' = 0$,
- slobodno oslonjene (SS_y) ivice: za $y = 0$ i $y = b$: $u = w = u' = 0$,
- uklještene (CC) ivice: $u = v = w = u' = v' = 0$.

Ploča je analizirana kako bi se ispitao uticaj graničnih uslova na sopstvene frekvencije kompozitnih ploča s prethodno ubačenom delaminacijom konstantne površine. Ploča je diskretizovana pomoću mreže od 6×6 slojevitih konačnih elemenata s devet čvorova, s redukovanom integracijom. Sračunate su sopstvene frekvencije za prva četiri tona oscilovanja neoštećenih i oštećenih ploča i upoređene su s rezultatima iz [13] u Tabeli 2.

From Table 1 it is obvious that the proposed model is capable to predict the fundamental frequencies of circular laminated composite plates, even by using the quadrilateral FE to describe the circular plate geometry. A slightly stiffer response (2–4%) is obtained for all stacking sequences. Natural frequencies obviously increase with the level of orthotropy.

Example 5.1.2. The second benchmark example [13] is concerned with an 8-layer square composite plate with symmetric (0/90/45/90)_s stacking sequence. Side length of the plate is $a = 250\text{mm}$, while overall plate thickness is $h = 2.12\text{mm}$. All laminas are of equal thickness. The following material parameters are assumed for orthotropic constitutive models of all laminas: $E_1 = 132\text{GPa}$, $E_2 = 5.35\text{GPa}$, $G_{12} = G_{13} = 2.79\text{GPa}$, $\nu_{12} = \nu_{13} = 0.291$, $\nu_{23} = 0.300$, $\rho = 1446.2\text{kg/m}^3$. Square delamination of side $a_{del} = a/2$ is prescribed in the mid-plane (between layers 4 and 5) in the centre of the plate. The boundary conditions are prescribed along boundary edges as follows:

- simply supported (SS_x) edges: at $x = 0$ and $x = a$: $v = w = v' = 0$,
- simply supported (SS_y) edges: at $y = 0$ and $y = b$: $u = w = u' = 0$,
- clamped (CC) edges: $u = v = w = u' = v' = 0$.

The plate is analyzed to check the influence of boundary conditions on fundamental frequencies of composite plates with previously imposed delaminated zone of constant area. The plate is discretized by 6×6 9-node layered finite elements with reduced integration. Natural frequencies for first 4 modes for intact and delaminated plates are calculated and compared with the results from [13] in Table 2.

Tabela 2. Sopstvene frekvencije (Hz) neoštećenih i oštećenih (0/90/45/90)_s kompozitnih ploča s različitim uslovima oslanjanja, za prva četiri tona oscilovanja

Table 2. Natural frequencies (Hz) for intact and delaminated (0/90/45/90)_s composite plates with different boundary conditions for first four modes

	Model	State	1	2	3	4
Simply Supported Plate	FSDT	Intact	164.37	404.38	492.29	658.40
		Damaged	161.58	348.27	371.19	637.48
	Present	Intact	169.81	409.78	504.22	672.69
		Damaged	167.04	347.88	374.62	611.08
Clamped Plate	FSDT	Intact	346.59	651.51	781.06	1017.20
		Damaged	334.67	579.43	653.25	851.27
	Present	Intact	346.81	643.44	777.93	982.16
		Damaged	316.88	529.34	554.81	783.80

Rezultati prikazani u Tabeli 2 potvrđuju da je predloženi model u stanju da precizno predvidi sopstvene frekvencije neoštećenih i oštećenih kompozitnih ploča. Redukcija sopstvene frekvencije usled prisustva delaminacija veća je u slučaju viših tonova oscilovanja, za oba razmatrana slučaja.

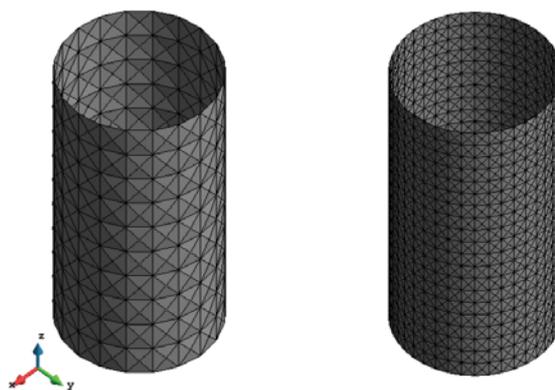
Primer 5.1.3. U trećem primeru razmatraju se cilindrične cross-ply laminatne kompozitne ljuske uklještene na oba kraja. Dužina razmatranih ljuski je $L=12\text{m}$, a poluprečnik $R=3\text{m}$. Ljuske su sačinjene od tri ortotropna sloja, pojedinačne debljine $h_k=0.02\text{m}$, tako da je ukupna debljina ljuske $h=0.06\text{m}$ (odnos dužina/poluprečnik je $L/R=4$, a odnos debljina/poluprečnik je $h/R=0.02$, što odgovara tankim i umereno dugim ljuskama). Za sve slojeve su pretpostavljeni materijalni parametri koji odgovaraju grafit-epoksidu [27]: $E_1 = 138\text{ GPa}$, $E_2 = E_3 = 8.96\text{ GPa}$, $G_{12} = G_{13} = 7.1\text{ GPa}$, $G_{23} = 3.45\text{ GPa}$, $\nu_{12} = 0.30$, $\rho = 1645\text{ kg/m}^3$. Primenjeni su slojeviti trougaoni konačni elementi s tri čvora. Ljuska je diskretizovana strukturiranim mrežom s dve različite gustine (Mreža 1 – 800 elemenata i Mreža 2 – 3200 elemenata, videti Sliku 3). Granični uslovi su zadati duž uklještenih ivica sprečavanjem svih generalisanih pomeranja u čvorovima. Vrednosti bezdimenzionalnih frekvencija $\Omega = \omega \cdot 100R(\rho/E_2)^{1/2}$ dobijene u analizama poređene su s rezultatima iz [46] dobijenim primenom 2D prstenastih konačnih elemenata, kao i s rezultatima iz [27] dobijenim primenom kontinualnih elemenata baziranih na dinamičkoj matrici krutosti. Vrednosti sopstvenih frekvencija grafički su prikazane na Slici 4, za različite šeme laminacije.

Ovaj primer jasno pokazuje da se prognošćenjem mreže dobijaju niže vrednosti sopstvenih frekvencija (konvergencija ka tačnom rešenju). Za 0/90/0 šemu laminacije dobijeno je odlično poklapanje u svim tonovima oscilovanja. Za 90/90/90 šemu laminacije dobijene su nešto niže vrednosti sopstvenih frekvencija u svim tonovima, zbog uticaja deformacije smicanja i idealizacija geometrije ljuske. Uzimanjem ovih napomena u obzir, predloženi model je u stanju da potpuno tačno predvidi sopstvene frekvencije laminatnih kompozitnih ljuski bez oštećenja.

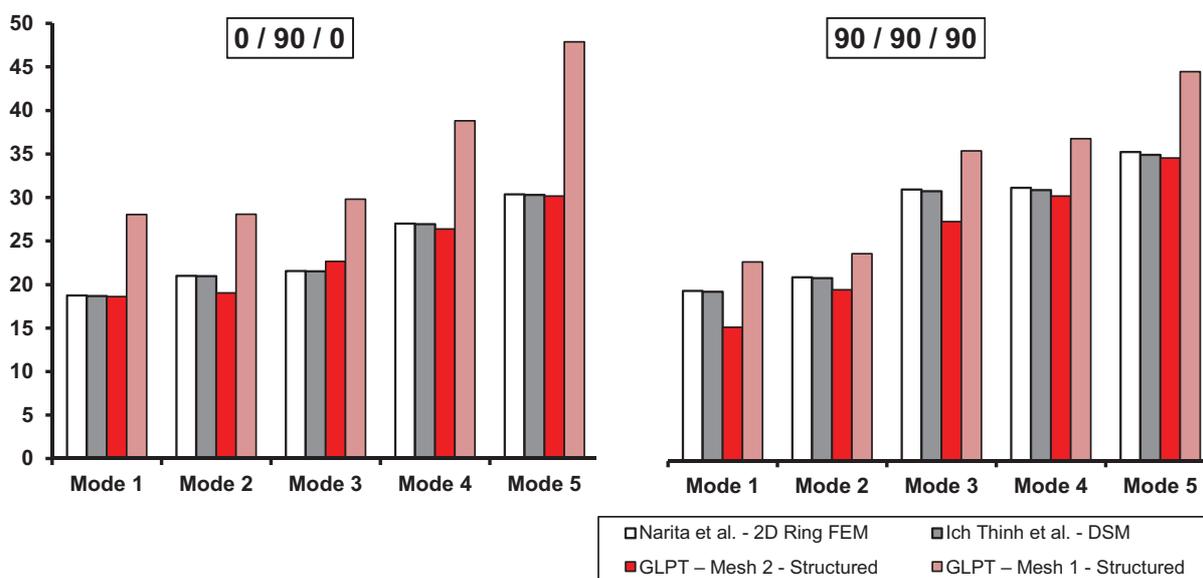
The results presented in Table 2 confirm that the proposed model is capable to accurately predict the fundamental frequencies of intact and delaminated composite plates. The reduction of the fundamental frequency caused by the presence of the delaminated zone is more pronounced for higher modes, for both examined cases.

Example 5.1.3. The third benchmark example is concerned with the cylindrical cross-ply laminated composite shells clamped at both ends. The length of the analyzed shells is $L=12\text{m}$, and the shell radius is $R=3\text{m}$. The shells are composed from three orthotropic layers, each of thickness $h_k=0.02\text{m}$, so the total shell thickness is $h=0.06\text{m}$ (length-to-radius ratio $L/R=4$ and thickness-to-radius ratio $h/R=0.02$, which is related to thin and moderately long shells). The material parameters (Graphite-Epoxy) for all layers are assumed as [27]: $E_1 = 138\text{ GPa}$, $E_2 = E_3 = 8.96\text{ GPa}$, $G_{12} = G_{13} = 7.1\text{ GPa}$, $G_{23} = 3.45\text{ GPa}$, $\nu_{12} = 0.30$, $\rho = 1645\text{ kg/m}^3$. Layered triangular 3-node elements are used. The shell is discretized using the structured mesh of two different densities (Mesh 1 – 800 elements and Mesh 2 - 3200 elements, see Figure 3). The boundary conditions are prescribed along clamped edges by constraining all generalized displacements in edge nodes. The values of non-dimensionalized frequency parameters $\Omega = \omega \cdot 100R(\rho/E_2)^{1/2}$ obtained from analyses are compared with the results by Narita et al. [46] using 2D ring FE model and Ich Thinh et al. [27] using continuous element constructed from the dynamic stiffness matrix. The results for fundamental frequencies are graphically interpreted in Figure 4, for different lamination schemes.

This example clearly shows that the mesh refinement leads to the lower values of frequency parameter (convergence to the exact solution). For the 0/90/0 lamination scheme, excellent agreement is obtained for all modes. For the 90/90/90 scheme, the slightly lower frequency parameters are obtained for all modes, because of the influence of the transverse shear deformation and the idealizations regarding the shell geometry. By taking these remarks into account, the presented model is fully capable to accurately predict the fundamental frequencies of intact laminated composite shells.



Slika 3. Strukturirane mreže trougaonih konačnih elemenata
Figure 3. Structured meshes of triangular finite elements



Slika 4. Poređenje bezdimenzionalnih frekvencija $\Omega = \omega \cdot 100R(\rho/E_2)^{1/2}$ cross-ply cilindričnih ljuski ukleštenih na oba kraja, dobijenih primenom različitih numeričkih modela
Figure 4. Comparison of frequency parameters $\Omega = \omega \cdot 100R(\rho/E_2)^{1/2}$ of cross-ply cylindrical shells clamped at both ends, obtained using different numerical models

5.2 Geometrijski nelinearna dinamička analiza

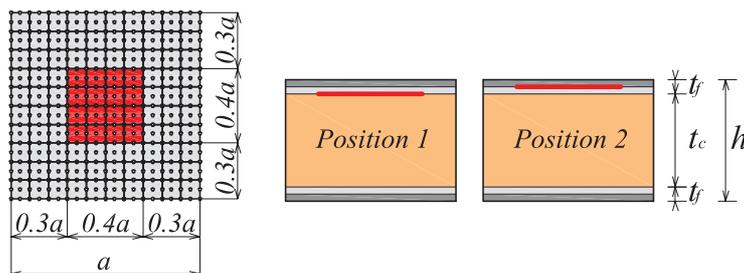
Primer 5.2.1. Poslednji primer ilustruje sposobnost predloženog modela za određivanje nezavisnog kretanja susednih slojeva u zoni delaminacija u sendvič-panelu. Numerički su ispitani linearni i geometrijski nelinearni dinamički odgovori sendvič-ploče s delaminacijom usled naglog eksponencijalnog opterećenja zbog eksplozije. Analizirana je petoslojna (0/90/jezgro/0/90) antisimetrična slobodno oslonjena (SS) kvadratna sendvič-ploča [6, 30]. Panel je napravljen od cross-ply obloge debljine t_f i mekog jezgra debljine t_c , gde je $t_c/t_f = 10$. Dužina strane ploče je $a = 250 \text{ mm}$, a njena debljina je $h = 2.50 \text{ mm}$ ($a/h = 100$). Obloga panela napravljena je od grafit-epoksida T300/934 sa sledećim mehaničkim karakteristikama: $E_{1,f} = 131 \text{ GPa}$, $E_{2,f} = E_{3,f} = 10.34 \text{ GPa}$,

5.2 Geometrically nonlinear transient analysis

Example 5.2.1. The final benchmark example illustrates the capability of the proposed model to represent the independent motions of adjacent delaminated interfaces in a sandwich panel. The linear and geometrically nonlinear transient responses of a delaminated sandwich plate under exponential blast pulse loading are investigated numerically. A five layer (0/90/core/0/90) anti-symmetric simply supported (SS) square sandwich plate [6, 30] is analyzed. The plate is composed from cross-ply face sheets, with thickness t_f and a soft core with thickness t_c , where $t_c/t_f = 10$. The side length of the plate is $a = 250 \text{ mm}$ and its height is $h = 2.50 \text{ mm}$ ($a/h = 100$). The face sheets are made of Graphite-Epoxy T300/934 with the following mechanical characteristics: $E_{1,f} = 131 \text{ GPa}$, $E_{2,f} = E_{3,f} = 10.34 \text{ GPa}$,

$G_{12,f} = G_{23,f} = 6.895 \text{ GPa}$, $G_{13,f} = 6.205 \text{ GPa}$, $\nu_{12,f} = \nu_{13,f} = 0.22$, $\nu_{23,f} = 0.49$, $\rho_f = 1627 \text{ kg/m}^3$. Izotropno meko jezgro napravljeno je od materijala sledećih karakteristika: $E_c = 6.89 \text{ MPa}$, $G_c = 6.895 \text{ MPa}$, $\nu_c = 0$, $\rho_c = 1550 \text{ kg/m}^3$. Ploča je diskretizovana pomoću mreže od 10×10 slojevitih konačnih elemenata sa devet čvorova s redukovanom integracijom. Jednako podeljeno poprečno opterećenje $q_0 = 1.0 \text{ kN/m}^2$ zadato je u vidu eksponencijalnog pulsa $q(t) = q_0 \cdot e^{-\alpha t}$, trajanja $T = 24 \text{ ms}$, gde je $\alpha = 150 \text{ s}^{-1}$ usvojeno kao fiktivni faktor prigušenja. Vremenski inkrement je $\Delta t = 0.8 \text{ ms}$. Bezdimenzionalni ugib sračunava se kao $w_0 = w \cdot E_{1,f} \cdot h^3 / q_0 / a^4$. Dinamički odgovor sračunat je za centralno pozicioniranu delaminaciju konstantne površine (videti Sliku 5) na pozicijama 1–2 i prikazan je na Slici 6.

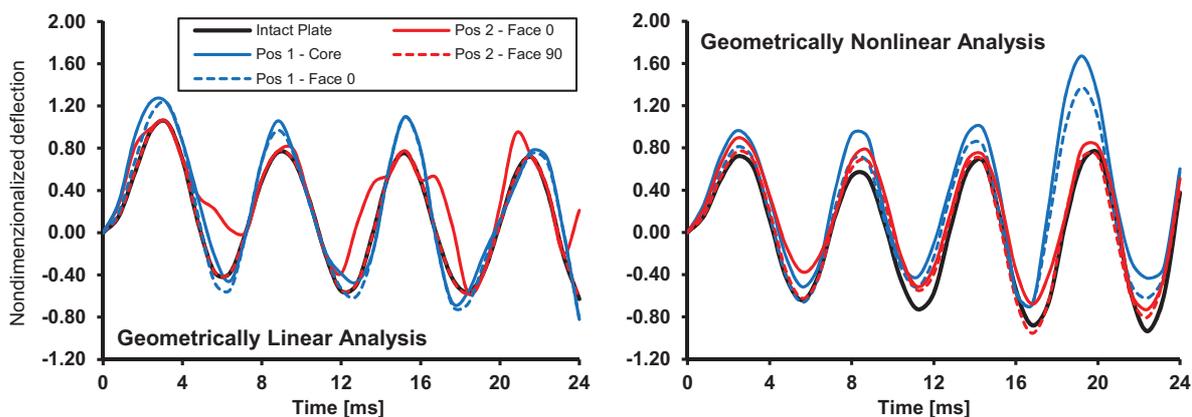
$G_{12,f} = G_{23,f} = 6.895 \text{ GPa}$, $G_{13,f} = 6.205 \text{ GPa}$, $\nu_{12,f} = \nu_{13,f} = 0.22$, $\nu_{23,f} = 0.49$, $\rho_f = 1627 \text{ kg/m}^3$. For the isotropic soft-core the following material parameters are adopted: $E_c = 6.89 \text{ MPa}$, $G_c = 6.895 \text{ MPa}$, $\nu_c = 0$, $\rho_c = 1550 \text{ kg/m}^3$. The plate is discretized using a 10×10 mesh of 9-node layered finite elements with reduced integration. Uniformly distributed transverse loading $q_0 = 1.0 \text{ kN/m}^2$ is prescribed as an exponential pulse $q(t) = q_0 \cdot e^{-\alpha t}$, with the duration of $T = 24 \text{ ms}$, using $\alpha = 150 \text{ s}^{-1}$ as a fictitious damping factor. The time increment is $\Delta t = 0.8 \text{ ms}$. The normalized centre transverse deflection is calculated as $w_0 = w \cdot E_{1,f} \cdot h^3 / q_0 / a^4$. The transient response is obtained for centrally located constant-area-delamination (see Figure 5) in positions 1-2 and plotted in Figure 6.



Slika 5. Sendvič-ploča s mekim jezgrom s delaminacijom na različitim pozicijama
Figure 5. Soft-core sandwich plate with different positions of an embedded delamination

Na Slici 6 ilustrovan je uticaj pozicije ranije ubačene delaminacije na rezultate dinamičkog proračuna sendvič-ploče sa oštećenjem. Sendvič-ploča veoma je osetljiva na oštećenje veze između obloge i jezgra, kada je opterećena eksponencijalnim opterećenjem usled eksplozije (plave linije na Slici 6). Ukoliko delaminacija postoji u okviru obloge panela, globalne amplitude pomeranja približno su iste kao i u slučaju neoštećene ploče, jer neoštećen deo ploče ima približno istu krutost na savijanja kao i neoštećena ploča (crvene linije na Slici 6). Razdvojeni segment osciluje lokalno svojom sopstvenom frekvencijom, izazivajući velike otvore prsline. U slučaju geometrijski nelinearne analize, dodatna krutost na savijanje dovodi do redukcije otvaranja prsline u poređenju s linearnom analizom.

Figure 6 illustrates the influence of the position of the previously prescribed delaminated zone on the results of transient analysis of the delaminated sandwich plate. The sandwich plate is highly vulnerable to face-core debonding when subjected to exponential blast pulse loading (blue lines in Figure 6). If the delamination occurs within the face sheets the global amplitudes are nearly the same as for the intact plate because the undamaged part has more or less the same bending stiffness as the intact plate (red lines in Figure 6). The delaminated segment oscillates locally with its local frequency, causing large crack opening displacements. In the geometrically nonlinear case the added bending stiffness leads to the reduction of the crack opening displacements as compared to the linear case.



Slika 6. Promena ugiba u centru dva susedna sloja sendvič-ploče kroz vreme, za različite položaje zone oštećenja
Figure 6. Temporal evolution of the central transverse deflection of two adjacent delaminated interfaces of a sandwich plate considering different positions of the delaminated area

6 ZAKLJUČCI

Na osnovu opšte laminatne teorije ploča izvedeni su slojeviti konačni elementi koji su u stanju da uvedu nezavisno kretanje razdvojenih slojeva. Međusobno prodiranje sloja u sloj sprečeno je uvođenjem kontaktnih uslova između pojedinih slojeva. Geometrijska nelinearnost je uzeta u obzir na osnovu Von Kármán-ovih pretpostavki. Predloženi numerički model primenjen je u numeričkoj analizi slobodnih vibracija i dinamičkog odgovora laminatnih kompozitnih i sendvič-ploča i ljuski s delaminacijama. Mnogim primerima ilustrovano je na koji način delaminacija utiče na fundamentalne dinamičke osobine laminatnih konstrukcija. U narednim radovima predloženi model će biti proširen kako bi se uzela u obzir propagacija delaminacije, pomoću proračuna brzine oslobađanja energije. Iz numeričke analize može se zaključiti:

1. Predloženi model blago (2–4%) precenjuje vrednosti sopstvenih frekvencija kružnih laminatnih kompozitnih ploča, za sve razmatrane šeme laminacije, zbog primene četvorougaoih konačnih elemenata za opisivanje kružne geometrije.

2. Predloženi model je u stanju da precizno predvodi sopstvene frekvencije neoštećenih ili oštećenih kompozitnih ploča. Uvođenjem smičuće deformacije u proračun smanjuju se sopstvene frekvencije neoštećenih i oštećenih kompozitnih ploča. Smanjenje frekvencije zbog prisustva delaminacija izraženije je u višim tonovima, za razmatrane uslove oslanjanja (CC i SS). Sopstvene frekvencije očigledno se povećavaju s porastom stepena ortotropije.

3. Progušćenjem mreže dobijaju se niže vrednosti sopstvenih frekvencija laminatnih kompozitnih cilindričnih ljuski (konvergencija ka tačnom rešenju). Za **0/90/0** šemu laminacije dobijeno je odlično poklapanje u svim tonovima, dok je za **90/90/90** šemu laminacije u svim tonovima dobijena nešto niža sopstvena frekvencija, zbog uticaja deformacije smicanja, kao i zbog idealizacija u pogledu geometrije ljuske.

4. Predloženi model je u stanju da precizno predvodi relativna pomeranja susednih slojeva u oštećenoj zoni. Na dinamički odgovor sendvič-ploča više utiče delaminacija koja se nalazi između mekog jezgra i obloge panela, u poređenju s delaminacijom koja se nalazi između pojedinih slojeva obloge panela. Ovo potvrđuje činjenicu da je čvrsta veza jezgra za oblogu panela od presudnog značaja u projektovanju sendvič-panela.

5. Ukoliko dođe do delaminacija u okviru obloge panela, u geometrijski linearnoj analizi odvojeni segment osciluje lokalno visokom frekvencijom i izaziva kompleksne kontaktne mehanizme između odvojenog sloja i neoštećenog ostatka ploče. U geometrijski nelinearnoj analizi ovaj složeni mehanizam ne postoji zbog prisustva dodatne krutosti na savijanje.

6 CONCLUSIONS

Layered finite plate elements, capable of incorporating the independent motion of delaminated interfaces between layers, have been derived based upon the Generalized Laminated Plate Theory. Interlaminar penetration between delaminated layers was prevented by considering contact conditions between the individual layers. Geometrical nonlinearity is accounted for based upon the Von Kármán assumptions. The proposed numerical model has been applied to the numerical analysis of the free vibrations and the transient response of delaminated composite and sandwich plates and shells. Through the variety of examples it is illustrated how the embedded delamination affect the fundamental dynamic properties of laminated structures. Future work includes the extension of the proposed model to account for propagation of the delamination using Energy Release Rate calculations. From the numerical analyses, the following conclusions are drawn:

1. The proposed model slightly (2-4%) overpredicts the natural frequencies of circular laminated composite plates, for all considered stacking sequences, because of the application of quadrilateral finite elements to describe the circular geometry.

2. The proposed model is capable to accurately predict the fundamental frequencies of intact and delaminated composite plates. The incorporation of the transverse shear deformation reduces the fundamental frequency both for the intact and delaminated composite plates. The reduction caused by the presence of the embedded delamination is more pronounced for higher modes, for examined types of boundary conditions (CC and SS). Natural frequencies obviously increase with the level of orthotropy.

3. The mesh refinement leads to the lower values of natural frequency of laminated composite cylindrical shells. For the **0/90/0** lamination scheme, excellent agreement is obtained for all modes, while for the **90/90/90** scheme, the slightly lower frequency parameters are obtained for all modes, because of the influence of the transverse shear deformation and the idealizations regarding the shell geometry.

4. The proposed model is capable to accurately predict the relative displacements of adjacent laminas in the damaged area. The transient response of sandwich plates is affected more if the delamination is positioned between the soft-core and the rigid face sheets as compared to the delamination within laminas of the face sheet, which confirms that the strong bonding of the soft core to the face sheet is the most critical aspect in the design of sandwich plates.

5. If delamination occurs within the rigid face sheet, when geometrically linear analysis is performed, the delaminated segment oscillates locally with a high frequency, causing complex contact closure and delamination mechanisms between the sheet layer and the intact rest of the plate. In the geometrically nonlinear analysis this complex mechanism is absent due to the added bending stiffness.

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REZIME

PRIMENA SLOJEVITIH KONAČNIH ELEMENATA U NUMERIČKOJ ANALIZI LAMINATNIH KOMPOZITNIH I SENDVIČ-PLOČA I LJUSKI S DELAMINACIJAMA

Dorđe VUKSANOVIĆ
Miroslav MARJANOVIĆ

Laminatni kompoziti su moderni materijali koji se široko primenjuju u mašinstvu i građevinarstvu. U ovom radu prikazani su odgovarajući moderni pristupi u numeričkoj analizi laminatnih kompozitnih i sendvič-ploča i ljuski s delaminacijom u pojedinim delovima konstrukcije. Za određivanje numeričkog rešenja različitih problema primenjeni su slojeviti konačni elementi bazirani na Reddy-evoj opštoj laminatnoj teoriji ploča. Nakon verifikacije postojećeg modela za konstrukcije bez oštećenja (primenom postojećih podataka iz literature), putem različitih numeričkih primera, analizirani su efekti veličine i položaja delaminacija na odgovor oštećenih laminatnih konstrukcija.

Ključne reči: Laminatni kompozit, Sendvič-ploča, Metod konačnih elemenata, Delaminacija, Kontakt

SUMMARY

APPLICATION OF LAYERED FINITE ELEMENTS IN THE NUMERICAL ANALYSIS OF LAMINATED COMPOSITE AND SANDWICH STRUCTURES WITH DELAMINATIONS

Djordje VUKSANOVIC
Miroslav MARJANOVIC

Laminar composites are modern engineering materials widely used in the mechanical and civil engineering. In the paper, some recent advances in a numerical analysis of laminated composite and sandwich plates and shells of different shapes, with existing zones of partial delamination, are presented. The layered finite elements, based on the extended version of the Generalized Laminated Plate Theory of Reddy, are applied for the numerical solution of several structural problems. After the verification of the proposed model for intact structures using the existing data from the literature, the effects of the size and the position of embedded delamination zones on the structural response of laminated structures are investigated numerically by means of a variety of numerical applications.

Key words: Laminar Composite, Sandwich Plate, Finite Element Method, Delamination, Contact

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